

APPENDIX 21-7

August 2008 Submittal

Additional and/or Revised
Intersection Analyses
Turning Movement Worksheets
Signal Timing
Existing Conditions
Future without Project
Future with Project

Appendix 21-7 - Intersection Analysis Summary

Intersection capacity and levels of service have been determined through the use of the latest version of the Highway Capacity Software (HCS+). Study area intersections located adjacent to significant roadway segments operating below the adopted LOS have been analyzed under existing traffic conditions, future without project and future with project to examine network operations during the PM peak hour. **Appendix 21-7** includes the following:

- A summary of the intersection analysis results provided in **Table 21-7**;
- Intersection turning movement worksheets - **Tables 21-7-A to 21-7-I**;
- Signal timing worksheets;
- Intersection Analysis Worksheets for Existing, Future without Project, Future with Project.

Table 21-7 Summary of the PM Peak Hour Study Intersection LOS							
Ref. No.	Intersection	Existing Traffic Control	Future without Project Traffic Control	Future with Project Traffic Control	Existing [Based upon geometry existing when the counts were collected]	Future without Project	Future with Project
1	SW 216 St and SW 177 Ave	Signalized	Signalized	Signalized	C	C	C
2	SW 200 St and SW 177 Ave	Signalized	Signalized	Signalized	E	C	C
3	SW 184 St and SW 177 Ave	Signalized	Signalized	Signalized	D	C	C
4	SW 152 St and SW 177 Ave	N/A	N/A	Signalized	N/A	N/A	B
5	SW 136 St and SW 177 Ave	N/A	N/A	Signalized	N/A	N/A	B
6	SW 88 St and SW 177 Ave	Signalized	Signalized	Signalized	D	C	C
7	SW 136 St and SW 157 Ave	Unsignalized	Unsignalized	Signalized	NBR, WBL only – no analysis	D/B	C
8	SW 152 St and SW 157 Ave	Unsignalized	Signalized	Signalized	C	C	D
9	SW 152 St and SW 147 Ave	Unsignalized	Signalized	Signalized	E	B	C
10	SW 152 St and SW 137 Ave	Signalized	Signalized	Signalized	F	F	E
11	SW 152 St and SW 117 Ave	Signalized	Signalized	Signalized	E	F	E
12	SW 152 St and W. HEFT Ramp	Signalized	Signalized	Signalized	D	C	C
13	SW 184 St and SW 147 Ave	Unsignalized	Signalized	Signalized	E	B	C
14	SW 184 St and SW 157 Ave	Unsignalized	Signalized	Signalized	C/D	C	C

The Future without Project intersection analyses incorporate recently completed improvements or those funded in TIP 2009 as outlined below:

- **SW 216 St and SW 177 Ave** – Geometric improvements completed by FDOT;
- **SW 200 St and SW 177 Ave** – Geometric improvements completed by FDOT;
- **SW 184 St and SW 177 Ave** – Geometric improvements completed by FDOT;
- **SW 88 St and SW 177 Ave** – Geometric improvements per TIP No. DT2496143;
- **SW 136 St and SW 157 Ave** – Geometric improvements per TIP No. PW20040354;
- **SW 152 St and SW 157 Ave** – Signal and new lane geometry completed by developer;
- **SW 152 St and SW 147 Ave** – Signalization completed by County;
- **SW 152 St and SW 117 Ave** – Geometric improvements completed by County;
- **SW 152 St and West HEFT Ramps** – Geometric improvements per TIP No. TP4060961;
- **SW 184 St and SW 147 Ave** – Geometric and signal improvements completed; additional lane geometry per TIP No. PW671572B;
- **SW 184 St and SW 157 Ave** – Geometric and signal improvements per TIP No. PW20040372.

The Future with Project intersection analyses incorporate additional improvements proposed by the Parkland DRI for following intersections:

- **SW 152 St and SW 177 Ave** – Lane widening and signalization;
- **SW 136 St and SW 177 Ave** – Lane widening and signalization;
- **SW 136 St and SW 157 Ave** – Lane widening and signalization;
- **SW 152 St and SW 157 Ave** – Lane widening;
- **SW 152 St and SW 147 Ave** – Lane widening;
- **SW 152 St and SW 137 Ave** – Overlap phases, restriping, lane widening (if ROW is available), signal optimization;
- **SW 152 St and SW 117 Ave** – Turn lane improvements, restriping and lane widening;
- **SW 152 St and West HEFT Ramps** – Restriping and lane widening;
- **SW 184 St and SW 157 Ave** – Lane widening;
- **SW 184 St and SW 147 Ave** – Lane widening.

**TABLE 21-7-A
PARKLAND DRI
SIGNALIZED INTERSECTION ANALYSIS
PROJECTED TURNING MOVEMENTS - PM PEAK HOUR**

8/25/2008

		PM PEAK HOUR															
No.	INTERSECTION	MVNT	12-12-06 2006 VOLUMES	FDOT PSCF	PEAK SEASON 2006 VOLUMES	GROWTH RATE TO 2018	2018 BKGD VOLUMES	COMMITTED PROJECT VOLUMES	FUTURE WITHOUT PROJECT	In = 2,288 Out = 1,201 PROJECT PRODUCTIONS	INBOUND AND OUTBOUND DIST.%	In = 745 Out = 1,241 PROJECT ATTRACTIONS	INBOUND AND OUTBOUND DIST.%	2018 TOTAL VOLUMES	EXISTING AS OF 2006 LANE GEOMETRY	WO PROJ FUTURE LANE GEOMETRY	WITH PROJ FUTURE LANE GEOMETRY
1	SW 177 Avenue and SW 200 Street Signalized	EB															
		LEFT	78	0.99	77	0.63%	83	0	83	0	0.00%	0	0.00%	83	1LTR	[Built as of 2008] 1L	1L
		THRU	103	0.99	102	0.63%	110	0	110	0	0.00%	0	0.00%	110			
		RIGHT	24	0.99	24	0.63%	26	0	26	0	0.00%	0	0.00%	26		1TR	1TR
		WB															
		LEFT	83	0.99	82	0.63%	89	0	89	0	0.00%	0	0.00%	89	1L	1L	1L
		THRU	118	0.99	117	0.63%	126	0	126	0	0.00%	0	0.00%	126		1T	1T
		RIGHT	129	0.99	128	0.63%	138	0	138	23	1.00%	7	1.00%	168	1TR	1R	1R
		NB															
		LEFT	65	0.99	64	0.63%	69	0	69	0	0.00%	0	0.00%	69	1L	1L	1L
		THRU	581	0.99	575	0.63%	620	55	675	46	2.00%	7	1.00%	728			
		RIGHT	99	0.99	98	0.63%	106	0	106	0	0.00%	0	0.00%	106	1TR	1TR	1TR
SB																	
LEFT	124	0.99	123	0.63%	132	0	132	12	1.00%	12	1.00%	156	1L	1L	1L		
THRU	643	0.99	637	0.63%	686	82	768	24	2.00%	12	1.00%	804					
RIGHT	45	0.99	45	0.63%	48	0	48	0	0.00%	0	0.00%	48	1TR	1TR	1TR		
		PM PEAK HOUR															
No.	INTERSECTION	MVNT	12-12-06 2006 VOLUMES	FDOT PSCF	PEAK SEASON 2006 VOLUMES	GROWTH RATE TO 2018	2018 BKGD VOLUMES	COMMITTED PROJECT VOLUMES	FUTURE WITHOUT PROJECT	In = 2,288 Out = 1,201 PROJECT PRODUCTIONS	INBOUND AND OUTBOUND DIST.%	In = 745 Out = 1,241 PROJECT ATTRACTIONS	INBOUND AND OUTBOUND DIST.%	2018 TOTAL VOLUMES	EXISTING AS OF 2006 LANE GEOMETRY	WO PROJ FUTURE LANE GEOMETRY	WITH PROJ FUTURE LANE GEOMETRY
2	SW 177 Avenue and SW 216 Street Signalized	EB															
		LEFT	49	0.99	49	0.63%	52	0	52	0	0.00%	0	0.00%	52	1LTR	[Built as of 2008] 1L	1L
		THRU	87	0.99	86	0.63%	93	0	93	0	0.00%	0	0.00%	93			
		RIGHT	76	0.99	75	0.63%	81	0	81	0	0.00%	0	0.00%	81		1TR	1TR
		WB															
		LEFT	45	0.99	45	0.63%	48	0	48	0	0.00%	0	0.00%	48	1LTR	1L	1L
		THRU	89	0.99	88	0.63%	95	0	95	0	0.00%	0	0.00%	95			
		RIGHT	59	0.99	58	0.63%	63	0	63	23	1.00%	0	0.00%	86		1TR	1TR
		NB															
		LEFT	29	0.99	29	0.63%	31	0	31	0	0.00%	0	0.00%	31	1L	1L	1L
		THRU	588	0.99	582	0.63%	628	55	683	23	1.00%	7	1.00%	713			
		RIGHT	48	0.99	48	0.63%	51	0	51	0	0.00%	0	0.00%	51	1TR	1TR	1TR
SB																	
LEFT	73	0.99	72	0.63%	78	0	78	12	1.00%	0	0.00%	90	1L	1L	1L		
THRU	685	0.99	678	0.63%	731	82	813	12	1.00%	12	1.00%	837					
RIGHT	57	0.99	56	0.63%	61	0	61	0	0.00%	0	0.00%	61	1TR	1TR	1TR		

BKGD= BACKGROUND
MVNT= MOVEMENT

**TABLE 21-7-B
PARKLAND DRI
SIGNALIZED INTERSECTION ANALYSIS
EXISTING AND PROJECTED TURNING MOVEMENTS - PM PEAK HOUR**

8/25/2008

		PM PEAK HOUR																
No.	INTERSECTION	MVNT	03-11-05 2005 VOLUMES	FDOT PSCF	PEAK SEASON 2005 VOLUMES	GROWTH RATE TO 2018	2018 BKGD VOLUMES	COMMITTED PROJECT VOLUMES	FUTURE WITHOUT PROJECT	In = 2,288 Out = 1,201 PROJECT PRODUCTIONS	INBOUND AND OUTBOUND DIST.%	In = 745 Out = 1,241 PROJECT ATTRACTIONS	INBOUND AND OUTBOUND DIST.%	2018 TOTAL VOLUMES	EXISTING AS OF 2005 LANE GEOMETRY	WO PROJ FUTURE LANE GEOMETRY	WITH PROJ FUTURE LANE GEOMETRY	
3	SW 184 Street and Krome Avenue Signalized	EB														[Built as of 2008] 1L	1L	
		LEFT	62	1.00	62	0.63%	67	0	67	0	0.00%	0	0.00%	67	1LTR			
		THRU	30	1.00	30	0.63%	33	0	33	0	0.00%	0	0.00%	33				
		RIGHT	7	1.00	7	0.63%	8	0	8	0	0.00%	0	0.00%	8			1TR	1TR
		WB																
		LEFT	212	1.00	212	0.63%	230	0	230	0	0.00%	0	0.00%	230	1LT	1L	1L	
		THRU	25	1.00	25	0.63%	27	0	27	0	0.00%	0	0.00%	27		1T	1T	
		RIGHT	99	1.00	99	0.63%	107	8	115	23	1.00%	7	1.00%	145		1R	1R	1R
		NB																
		LEFT	3	1.00	3	0.63%	3	0	3	0	0.00%	0	0.00%	3	1L	1L	1L	
		THRU	609	1.00	609	0.63%	661	50	711	69	3.00%	15	2.00%	795		1T	1T	
		RIGHT	196	1.00	196	0.63%	213	5	218	0	0.00%	0	0.00%	218		1TR	1R	1R
SB																		
LEFT	156	1.00	156	0.63%	169	10	179	12	1.00%	12	1.00%	203	1L	1L	1L			
THRU	688	1.00	688	0.63%	747	82	829	36	3.00%	25	2.00%	890						
RIGHT	26	1.00	26	0.63%	28	0	28	0	0.00%	0	0.00%	28		1TR	1TR	1TR		

BKGD= BACKGROUND

MVNT= MOVEMENT

**TABLE 21-7-C
PARKLAND DRI
SIGNALIZED INTERSECTION ANALYSIS
PROJECTED TURNING MOVEMENTS - PM PEAK HOUR**

8/25/2008

		PM PEAK HOUR															
No.	INTERSECTION	MVNT	3-11-05 2005 VOLUMES	FDOT PSCF	PEAK SEASON 2005 VOLUMES	GROWTH RATE TO 2018	2018 BKGD VOLUMES	COMMITTED PROJECT VOLUMES	FUTURE WITHOUT PROJECT	In = 2,288 Out = 1,201 PROJECT PRODUCTIONS	INBOUND AND OUTBOUND DIST.%	In = 745 Out = 1,241 PROJECT ATTRACTIONS	INBOUND AND OUTBOUND DIST.%	2018 TOTAL VOLUMES	WITH PROJ FUTURE LANE GEOMETRY	WITH PROJ FUTURE LANE GEOMETRY	
4	SW 177 Avenue and SW 136 Street Does not exist To be built and signalized by Parkland	EB													Before Widening of SW 177 Ave	With Widening of SW 177 Ave	
		LEFT	0	1.00	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			
		THRU	0	1.00	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			
		RIGHT	0	1.00	0	0.63%	0	0	0	0	0	0.00%	0	0.00%	0		
		WB															
		LEFT	0	1.00	0	0.63%	0	0	0	0	0	0.00%	0	0.00%	0	1L	1L
		THRU	0	1.00	0	0.63%	0	0	0	0	0	0.00%	0	0.00%	0		
		RIGHT	0	1.00	0	0.63%	0	0	0	108	9.00%	62	5.00%	170	1R	1R	
		NB															
		LEFT	0	1.00	0	0.63%	0	0	0	0	0	0.00%	0	0.00%	0		
		THRU	770	1.00	770	0.63%	836	58	894	60	5.00%	25	2.00%	979	1T	2T	
		RIGHT	0	1.00	0	0.63%	0	0	0	0	0	0.00%	0	0.00%	0	1R	1R
SB																	
LEFT	0	1.00	0	0.63%	0	0	0	206	9.00%	37	5.00%	243	1L	1L			
THRU	870	1.00	870	0.63%	944	92	1036	114	5.00%	15	2.00%	1165	1T	2T			
RIGHT	0	1.00	0	0.63%	0	0	0	0	0	0.00%	0	0.00%	0				
		PM PEAK HOUR															
No.	INTERSECTION	MVNT	03-11-05 2005 VOLUMES	FDOT PSCF	PEAK SEASON 2005 VOLUMES	GROWTH RATE TO 2018	2018 BKGD VOLUMES	COMMITTED PROJECT VOLUMES	FUTURE WITHOUT PROJECT	In = 2,288 Out = 1,201 PROJECT PRODUCTIONS	INBOUND AND OUTBOUND DIST.%	In = 745 Out = 1,241 PROJECT ATTRACTIONS	INBOUND AND OUTBOUND DIST.%	2018 TOTAL VOLUMES	WITH PROJ FUTURE LANE GEOMETRY	WITH PROJ FUTURE LANE GEOMETRY	
5	SW 177 Avenue and SW 152 Street Does not exist To be built and signalized by Parkland	EB													Before Widening of SW 177 Ave	With Widening of SW 177 Ave	
		LEFT	0	1.00	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			
		THRU	0	1.00	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			
		RIGHT	0	1.00	0	0.63%	0	0	0	0	0	0.00%	0	0.00%	0		
		WB															
		LEFT	0	1.00	0	0.63%	0	0	0	48	4.00%	37	3.00%	85	1L	1L	
		THRU	0	1.00	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			
		RIGHT	0	1.00	0	0.63%	0	0	0	60	5.00%	25	2.00%	85	1R	1R	
		NB															
		LEFT	0	1.00	0	0.63%	0	0	0	0	0	0.00%	0	0.00%	0		
		THRU	770	1.00	770	0.63%	836	58	894	0	0.00%	0	0.00%	894	1T	2T	
		RIGHT	0	1.00	0	0.63%	0	0	0	92	4.00%	22	3.00%	114	1R	1R	
SB																	
LEFT	0	1.00	0	0.63%	0	0	0	114	5.00%	15	2.00%	129	1L	1L			
THRU	870	1.00	870	0.63%	944	92	1036	0	0.00%	0	0.00%	1036	1T	2T			
RIGHT	0	1.00	0	0.63%	0	0	0	0	0	0.00%	0	0.00%	0				

BKGD= BACKGROUND
MVNT= MOVEMENT

**TABLE 21-7-D
PARKLAND DRI
SIGNALIZED INTERSECTION ANALYSIS
EXISTING AND PROJECTED TURNING MOVEMENTS - PM PEAK HOUR**

8/25/2008

PM PEAK HOUR																	
No.	INTERSECTION	MVNT	03-11-05 2005 VOLUMES	FDOT PSCF	PEAK SEASON 2005 VOLUMES	GROWTH RATE TO 2018	2018 BKGD VOLUMES	COMMITTED PROJECT VOLUMES	FUTURE WITHOUT PROJECT	In = 2,288 Out = 1,201 PROJECT PRODUCTIONS	INBOUND AND OUTBOUND DIST. %	In = 745 Out = 1,241 PROJECT ATTRACTIONS	INBOUND AND OUTBOUND DIST. %	2018 TOTAL VOLUMES	EXISTING AS OF 2005 LANE GEOMETRY	WO PROJ FUTURE LANE GEOMETRY	WITH PROJ FUTURE LANE GEOMETRY
6	SW 88 Street and SW 177 Avenue Signalized	EB															
		LEFT	11	1.00	11	0.63%	12	0	12	0	0.00%	0	0.00%	12	1LTR	[1]	1LTR
		THRU	12	1.00	12	0.63%	13	0	13	0	0.00%	0	0.00%	13			
		RIGHT	5	1.00	5	0.63%	5	0	5	0	0.00%	0	0.00%	5			
		WB															
		LEFT	319	1.00	319	0.63%	346	51	397	46	2.00%	15	2.00%	458	1LT	1L	1L
		THRU	3	1.00	3	0.63%	3	0	3	0	0.00%	0	0.00%	3		1T	1T
		RIGHT	159	1.00	159	0.63%	173	64	237	0	0.00%	0	0.00%	237	1R	1R	1R
		NB															
		LEFT	3	1.00	3	0.63%	3	0	3	0	0.00%	0	0.00%	3	1L	1L	1L
		THRU	399	1.00	399	0.63%	433	10	443	144	12.00%	62	5.00%	649	1T	2T	2T
		RIGHT	392	1.00	392	0.63%	425	48	473	24	2.00%	25	2.00%	522	1R	1R	1R
		SB															
		LEFT	342	1.00	342	0.63%	371	65	436	0	0.00%	0	0.00%	436	1L	1L	1L
		THRU	472	1.00	472	0.63%	512	41	553	275	12.00%	37	5.00%	865	1T	1T	1T
		RIGHT	12	1.00	12	0.63%	13	0	13	0	0.00%	0	0.00%	13	1R	1TR	1TR

BKGD= BACKGROUND

MVNT= MOVEMENT

[1] Includes programmed improvements from TIP No. DT2496143.

**TABLE 21-7-E
PARKLAND DRI
SIGNALIZED INTERSECTION ANALYSIS
EXISTING AND PROJECTED TURNING MOVEMENTS - PM PEAK HOUR**

8/25/2008

PM PEAK HOUR																	
No.	INTERSECTION	MVNT	12-07-05 2005 VOLUMES	FDOT PSCF	PEAK SEASON 2005 VOLUMES	GROWTH RATE TO 2018	2018 BKGD VOLUMES	COMMITTED PROJECT VOLUMES	FUTURE WITHOUT PROJECT	In = 2,288 Out = 1,201 PROJECT PRODUCTIONS	INBOUND AND OUTBOUND DIST.%	In = 745 Out = 1,241 PROJECT ATTRACTIONS	INBOUND AND OUTBOUND DIST.%	2018 TOTAL VOLUMES	EXISTING AS OF 2005 LANE GEOMETRY	WO PROJ FUTURE LANE GEOMETRY	WITH PROJ FUTURE LANE GEOMETRY
7	SW 136 Street and SW 157 Avenue Unsignalized Signalized by Parkland	EB														[1]	
		LEFT	0	0.99	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			
		THRU	0	0.99	0	0.63%	0	50	50	192	16.00%	248	20.00%	490		2T	2T
		RIGHT	0	0.99	0	0.63%	0	57	57	0	0.00%	0	0.00%	57		1R	1R
		WB															
		LEFT	459	0.99	454	0.63%	493	10	503	0	0.00%	0	0.00%	503	1L	1L	1L
		THRU	0	0.99	0	0.63%	0	44	44	366	16.00%	149	20.00%	559		2T	2T
		RIGHT	0	0.99	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			
		NB															
		LEFT	0	0.99	0	0.63%	0	55	55	114	5.00%	52	7.00%	222		1L	1L
		THRU	0	0.99	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			
		RIGHT	189	0.99	187	0.63%	203	16	219	0	0.00%	0	0.00%	219	1R	2R	2R
		SB															
		LEFT	0	0.99	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			
		THRU	0	0.99	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			
		RIGHT	0	0.99	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			

BKGD= BACKGROUND

MVNT= MOVEMENT

[1] Includes programmed improvements from TIP NO. PW20040346 and TIP NO. PW20040354.

**TABLE 21-7-F
PARKLAND DRI
SIGNALIZED INTERSECTION ANALYSIS
EXISTING AND PROJECTED TURNING MOVEMENTS - PM PEAK HOUR**

8/25/2008

PM PEAK HOUR																	
No.	INTERSECTION	MVNT	12-08-05 2005 VOLUMES	FDOT PSCF	PEAK SEASON 2005 VOLUMES	GROWTH RATE TO 2018	2018 BKGD VOLUMES	COMMITTED PROJECT VOLUMES	FUTURE WITHOUT PROJECT	In = 2,288 Out = 1,201 PROJECT PRODUCTIONS	INBOUND AND OUTBOUND DIST. %	In = 745 Out = 1,241 PROJECT ATTRACTIONS	INBOUND AND OUTBOUND DIST. %	2018 TOTAL VOLUMES	EXISTING AS OF 2005 LANE GEOMETRY	WO PROJ FUTURE LANE GEOMETRY	WITH PROJ FUTURE LANE GEOMETRY
8	SW 152 Street and SW 157 Avenue Unsignalized in 2005 Signalized in 2008	EB														[Built as of 2008]	[1]
		LEFT	6	0.99	6	0.63%	6	9	15	60	5.00%	124	10.00%	199	1LTR	1L	1L
		THRU	77	0.99	76	0.63%	83	7	90	312	26.00%	434	35.00%	837			2T
		RIGHT	23	0.99	23	0.63%	25	0	25	180	15.00%	62	5.00%	267		1TR	1TR
		WB															
		LEFT	67	0.99	66	0.63%	72	4	76	0	0.00%	0	0.00%	76	1LTR	1L	1L
		THRU	121	0.99	120	0.63%	130	7	137	595	26.00%	238	32.00%	970		1T	1T
		RIGHT	119	0.99	118	0.63%	128	55	183	0	0.00%	0	0.00%	183		1R	1TR
		NB															
		LEFT	39	0.99	39	0.63%	42	0	42	343	15.00%	37	5.00%	422	1LT	1L	1L
		THRU	86	0.99	85	0.63%	92	7	99	0	0.00%	0	0.00%	99		1T	1T
		RIGHT	70	0.99	69	0.63%	75	18	93	0	0.00%	0	0.00%	93	1TR	1TR	1TR
		SB															
		LEFT	186	1.03	192	0.63%	208	34	242	0	0.00%	0	0.00%	242	1LT	1L	1L
		THRU	165	1.03	170	0.63%	184	23	207	0	0.00%	0	0.00%	207		1T	1T
		RIGHT	28	1.03	29	0.63%	31	10	41	114	5.00%	75	10.00%	230	1TR	1TR	1TR

BKGD= BACKGROUND

MVNT= MOVEMENT

[1] Proposed lane widening by Parkland on SW 152 Street which expands the EB and WB intersection approaches to SW 157 Avenue.

**TABLE 21-7-G
PARKLAND DRI
SIGNALIZED INTERSECTION ANALYSIS
EXISTING AND PROJECTED TURNING MOVEMENTS - PM PEAK HOUR**

8/25/2008

		PM PEAK HOUR															
No.	INTERSECTION	MVNT	12-09-05 2005 VOLUMES	FDOT PSCF	PEAK SEASON 2005 VOLUMES	GROWTH RATE TO 2018	2018 BKGD VOLUMES	COMMITTED PROJECT VOLUMES	FUTURE WITHOUT PROJECT	In = 2,288 Out = 1,201 PROJECT PRODUCTIONS	INBOUND AND OUTBOUND DIST.%	In = 745 Out = 1,241 PROJECT ATTRACTIONS	INBOUND AND OUTBOUND DIST.%	2018 TOTAL VOLUMES	EXISTING AS OF 2005 LANE GEOMETRY	WO PROJ FUTURE LANE GEOMETRY	WITH PROJ FUTURE LANE GEOMETRY
9	SW 152 Street and SW 147 Avenue Unsignalized in 2005 Signalized in 2008	EB															
		LEFT	0	0.99	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			
		THRU	507	0.99	502	0.63%	545	59	604	312	26.00%	372	30.00%	1289	1T	1T	2T
		RIGHT	25	0.99	25	0.63%	27	0	27	0	0.00%	25	2.00%	52	1TR	1TR	1TR
		WB															
		LEFT	427	0.99	423	0.63%	459	17	476	0	0.00%	0	0.00%	476	1L	1L	1L
		THRU	723	0.99	716	0.63%	777	66	843	595	26.00%	209	28.00%	1646	2T	2T	2T
		RIGHT	0	0.99	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			
		NB															
		LEFT	25	0.99	25	0.63%	27	0	27	0	0.00%	15	2.00%	42	1L	1L	1L
		THRU	0	0.99	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			
		RIGHT	303	0.99	300	0.63%	325	20	345	0	0.00%	0	0.00%	345	1R	1R	1R
		SB															
		LEFT	0	0.99	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			
		THRU	0	0.99	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			
		RIGHT	0	0.99	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			
		PM PEAK HOUR															
No.	INTERSECTION	MVNT	03-11-05 2005 VOLUMES	FDOT PSCF	PEAK SEASON 2005 VOLUMES	GROWTH RATE TO 2018	2018 BKGD VOLUMES	COMMITTED PROJECT VOLUMES	FUTURE WITHOUT PROJECT	In = 2,288 Out = 1,201 PROJECT PRODUCTIONS	INBOUND AND OUTBOUND DIST.%	In = 745 Out = 1,241 PROJECT ATTRACTIONS	INBOUND AND OUTBOUND DIST.%	2018 TOTAL VOLUMES	EXISTING AS OF 2005 LANE GEOMETRY	WO PROJ FUTURE LANE GEOMETRY	WITH PROJ FUTURE LANE GEOMETRY
10	SW 152 Street and SW 137 Avenue Signalized	EB															
		LEFT	289	1.00	289	0.63%	314	0	314	0	0.00%	25	2.00%	339	2L	[Built as of 2008] 2L	[1] 2L
		THRU	606	1.00	606	0.63%	658	79	737	276	23.00%	236	19.00%	1249	2T	2T	3T
		RIGHT	114	1.00	114	0.63%	124	0	124	36	3.00%	87	7.00%	247	1R	1TR	1R
		WB															
		LEFT	582	1.00	582	0.63%	632	37	669	0	0.00%	0	0.00%	669	2L	2L	2L
		THRU	1086	1.00	1,086	0.63%	1178	63	1241	526	23.00%	142	19.00%	1909	2T	2T	3T
		RIGHT	555	1.00	555	0.63%	602	183	785	0	0.00%	0	0.00%	785	1R	1R	1R
		NB															
		LEFT	294	1.00	294	0.63%	319	0	319	69	3.00%	52	7.00%	440	2L	2L	2L
		THRU	732	1.00	732	0.63%	794	138	932	0	0.00%	0	0.00%	932	2T	2T	2T
		RIGHT	269	1.00	269	0.63%	292	153	445	0	0.00%	0	0.00%	445	1R	1R	1R
		SB															
		LEFT	532	1.00	532	0.63%	577	20	597	0	0.00%	0	0.00%	597	2L	2L	2L
		THRU	1022	1.00	1,022	0.63%	1109	136	1245	0	0.00%	0	0.00%	1245	2T	2T	3T
		RIGHT	349	1.00	349	0.63%	379	130	509	0	0.00%	15	2.00%	524	1R	1R	1R

BKGD= BACKGROUND

MVNT= MOVEMENT

[1] Proposed improvements include restriping the existing SB, EB and WB right turn lane into the third through lane (the alignment to the third receiving lane is already in place).
Further improvements include the addition of an exclusive SB, EB and WB right turn lane if ROW is available.

**TABLE 21-7-H
PARKLAND DRI
SIGNALIZED INTERSECTION ANALYSIS
EXISTING AND PROJECTED TURNING MOVEMENTS - PM PEAK HOUR**

8/27/2008

		PM PEAK HOUR															
No.	INTERSECTION	MVNT	12-09-05 2005 VOLUMES	FDOT PSCF	PEAK SEASON 2005 VOLUMES	GROWTH RATE TO 2018	2018 BKGD VOLUMES	COMMITTED PROJECT VOLUMES	FUTURE WITHOUT PROJECT	In = 2,288 Out = 1,201 PROJECT PRODUCTIONS	INBOUND AND OUTBOUND DIST.%	In = 745 Out = 1,241 PROJECT ATTRACTIONS	INBOUND AND OUTBOUND DIST.%	2018 TOTAL VOLUMES	EXISTING AS OF 2005 LANE GEOMETRY	WO PROJ FUTURE LANE GEOMETRY	WITH PROJ FUTURE LANE GEOMETRY
11	SW 152 Street and SW 117 Avenue Signalized	EB															
		LEFT	318	0.99	315	0.63%	342	45	387	96	8.00%	37	3.00%	520	1L	[1] 2L	[2] 2L
		THRU	1277	0.99	1,264	0.63%	1372	266	1638	144	12.00%	161	13.00%	1943	2T	1T	2T
		RIGHT	210	0.99	208	0.63%	226	45	271	24	2.00%	25	2.00%	320	1TR	1TR	1TR
		WB															
		LEFT	37	0.99	37	0.63%	40	0	40	0	0.00%	0	0.00%	40	1L	1L	1L
		THRU	1090	0.99	1,079	0.63%	1171	355	1526	275	12.00%	97	13.00%	1897	2T	2T	3T
		RIGHT	18	0.99	18	0.63%	19	0	19	0	0.00%	0	0.00%	19	1R	1R	1R
		NB															
		LEFT	223	0.99	221	0.63%	240	45	285	46	2.00%	15	2.00%	346	1L	1L	1L
		THRU	314	0.99	311	0.63%	337	0	337	0	0.00%	0	0.00%	337	1T	2T	2T
		RIGHT	76	0.99	75	0.63%	82	0	82	0	0.00%	0	0.00%	82	1R	1R	1R
		SB															
		LEFT	81	0.99	80	0.63%	87	0	87	0	0.00%	0	0.00%	87	1L	1L	1L
		THRU	450	0.99	446	0.63%	483	0	483	0	0.00%	0	0.00%	483	1T	1TR	1TR
		RIGHT	936	0.99	927	0.63%	1005	50	1055	183	8.00%	22	3.00%	1260	1R	1R	1R
		PM PEAK HOUR															
No.	INTERSECTION	MVNT	12-09-05 2005 VOLUMES	FDOT PSCF	PEAK SEASON 2005 VOLUMES	GROWTH RATE TO 2018	2018 BKGD VOLUMES	COMMITTED PROJECT VOLUMES	FUTURE WITHOUT PROJECT	In = 2,288 Out = 1,201 PROJECT PRODUCTIONS	INBOUND AND OUTBOUND DIST.%	In = 745 Out = 1,241 PROJECT ATTRACTIONS	INBOUND AND OUTBOUND DIST.%	2018 TOTAL VOLUMES	EXISTING AS OF 2005 LANE GEOMETRY	WO PROJ FUTURE LANE GEOMETRY	WITH PROJ FUTURE LANE GEOMETRY
12	SW 152 Street and SR 821 West Ramps SB Off and SB On Signalized	EB															
		LEFT	0	0.99	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			
		THRU	1156	0.99	1,144	0.63%	1242	211	1453	0	0.00%	74	6.00%	1527	2T	2T	3T
		RIGHT	310	0.99	307	0.63%	333	55	388	36	3.00%	50	4.00%	474	1R	1R	1R
		WB															
		LEFT	195	0.99	193	0.63%	209	0	209	0	0.00%	0	0.00%	209	1L	1L	1L
		THRU	1026	0.99	1,016	0.63%	1102	240	1342	69	3.00%	75	10.00%	1485	2T	2T	3T
		RIGHT	0	0.99	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			
		NB															
		LEFT	0	0.99	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			
		THRU	0	0.99	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			
		RIGHT	0	0.99	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			
		SB															
		LEFT	392	0.99	388	0.63%	421	0	421	0	0.00%	0	0.00%	421	1L	1L	1L
		THRU	100	0.99	99	0.63%	107	0	107	0	0.00%	0	0.00%	107	1LT	1LT	1LT
		RIGHT	124	0.99	123	0.63%	133	115	248	206	9.00%	22	3.00%	476	1R	1R	1R

BKGD= BACKGROUND

MVNT= MOVEMENT

[1] Built as of 2008 plus programmed from TIP No. TP4060961.

[2] Proposed restriping and lane widening in coordination with Miami-Dade County and Florida's Turnpike to create the third WB and EB through lane.

**TABLE 21-7-I
PARKLAND DRI
SIGNALIZED INTERSECTION ANALYSIS
EXISTING AND PROJECTED TURNING MOVEMENTS - PM PEAK HOUR**

8/27/2008

PM PEAK HOUR																	
No.	INTERSECTION	MVNT	12-08-05 2005 VOLUMES	FDOT PSCF	PEAK SEASON 2005 VOLUMES	GROWTH RATE TO 2018	2018 BKGD VOLUMES	COMMITTED PROJECT VOLUMES	FUTURE WITHOUT PROJECT	In = 2,288 Out = 1,201 PROJECT PRODUCTIONS	INBOUND AND OUTBOUND DIST.%	In = 745 Out = 1,241 PROJECT ATTRACTIONS	INBOUND AND OUTBOUND DIST.%	2018 TOTAL VOLUMES	EXISTING AS OF 2005 LANE GEOMETRY	WO PROJ FUTURE LANE GEOMETRY	WITH PROJ FUTURE LANE GEOMETRY
13	SW 184 Street and SW 147 Avenue Unsignalized in 2005 Signalized in 2008	EB														[1]	
		LEFT	43	0.99	43	0.63%	46	0	46	12	1.00%	0	0.00%	58		1L	1L
		THRU	288	0.99	285	0.63%	309	17	326	156	13.00%	50	4.00%	532	1LTR	1T	1T
		RIGHT	81	0.99	80	0.63%	87	0	87	12	1.00%	12	1.00%	111		1TR	1TR
		WB														[1]	
		LEFT	19	0.99	19	0.63%	20	0	20	0	0.00%	0	0.00%	20		1L	1L
		THRU	112	0.99	111	0.63%	120	18	138	275	12.00%	30	4.00%	442	1LTR	1T	1T
		RIGHT	66	0.99	65	0.63%	71	17	88	0	0.00%	0	0.00%	88		1R	1TR
		NB														[Built as of 2008]	
		LEFT	80	0.99	79	0.63%	86	0	86	23	1.00%	7	1.00%	116		1L	1L
		THRU	246	0.99	244	0.63%	264	0	264	0	0.00%	0	0.00%	264	1LTR		
		RIGHT	49	0.99	49	0.63%	53	18	71	0	0.00%	0	0.00%	71		1TR	1TR
		SB														[Built as of 2008]	
		LEFT	78	0.99	77	0.63%	84	0	84	0	0.00%	0	0.00%	84		1L	1L
		THRU	201	0.99	199	0.63%	216	58	274	0	0.00%	0	0.00%	274	1LTR		
		RIGHT	27	0.99	27	0.63%	29	0	29	23	1.00%	0	0.00%	52		1TR	1TR
PM PEAK HOUR																	
No.	INTERSECTION	MVNT	12-08-05 2005 VOLUMES	FDOT PSCF	PEAK SEASON 2005 VOLUMES	GROWTH RATE TO 2018	2018 BKGD VOLUMES	COMMITTED PROJECT VOLUMES	FUTURE WITHOUT PROJECT	In = 2,288 Out = 1,201 PROJECT PRODUCTIONS	INBOUND AND OUTBOUND DIST.%	In = 745 Out = 1,241 PROJECT ATTRACTIONS	INBOUND AND OUTBOUND DIST.%	2018 TOTAL VOLUMES	EXISTING AS OF 2005 LANE GEOMETRY	WO PROJ FUTURE LANE GEOMETRY	WITH PROJ FUTURE LANE GEOMETRY
14	SW 184 Street and SW 157 Avenue Unsignalized	EB														[2]	[3]
		LEFT	112	0.99	111	0.63%	120	0	120	0	0.00%	0	0.00%	120			1L
		THRU	335	0.99	332	0.63%	360	0	360	12	1.00%	12	1.00%	384	1LTR	1LTR	1T
		RIGHT	0	0.99	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			1TR
		WB															
		LEFT	0	0.99	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			1L
		THRU	159	0.99	157	0.63%	171	0	171	23	1.00%	7	1.00%	201	1LTR	1LTR	1T
		RIGHT	75	0.99	74	0.63%	81	26	107	343	15.00%	37	5.00%	487			1R
		NB															
		LEFT	0	0.99	0	0.63%	0	0	0	0	0.00%	0	0.00%	0			
		THRU	1	0.99	1	0.63%	1	0	1	0	0.00%	0	0.00%	1	1LTR	1LTR	1LTR
		RIGHT	1	0.99	1	0.63%	1	3	4	0	0.00%	0	0.00%	4			
		SB															
		LEFT	86	0.99	85	0.63%	92	14	106	180	15.00%	62	5.00%	348		1L	1L
		THRU	0	0.99	0	0.63%	0	2	2	0	0.00%	0	0.00%	2	1LTR	1T	1T
		RIGHT	156	0.99	154	0.63%	168	10	178	0	0.00%	0	0.00%	178		1R	1R

BKGD= BACKGROUND
MVNT= MOVEMENT

- [1] Include signalization and geometric improvements built as of 2008 and includes programmed improvements from TIP NO. PW671572B.
[2] Includes programmed improvements from TIP NO. PW20040372.
[3] Includes the proposed lane widening on SW 184 Street by Parkland which expands the EB and WB intersection approaches at SW 157 Avenue.

Signal Timing

4784

TIMING DATA FOR 4784 KROME AVE & SW 200 ST (SEC: 238 TYPE: SA)

PAT	OF	NSG	G	Y	R	EWG	Y	R	NSL	Y	S	Y	M	CYC
1	T	0	40	1	4	1	18	4	1	7	3	7	79	DAY 0/8
2	T	0	40	1	4	1	15	4	1	7	3	6	76	NITE 8/0
3	T	0	45	1	4	1	15	4	1	7	3	7	81	EARLY AM PEA
4	T	0	45	1	4	1	15	4	1	7	3	7	81	EARLY AFT. 0
5	T	0	40	1	4	1	18	4	1	7	3	7	79	LATE PM / EV
6	T	0	45	1	4	1	15	4	1	7	3	7	81	SCHOOL IN 0/
7	T	0	40	1	4	1	18	4	1	7	3	7	79	SCH. OUT 0/8
8	T	0	45	1	4	1	15	4	1	7	3	7	81	SCHOOL OUT 0
10	T	0	40	1	4	1	18	4	1	7	3	7	79	PRE-AM PEAK
14	T	0	45	1	4	1	15	4	1	7	3	7	81	AFTERNOON 0
24	T	0	44	1	4	1	15	4	1	7	3	7	80	RECALL TEST
MIN: 40 10 6														

4787

TIMING DATA FOR 4787 KROME AVE & SW 216 ST (SEC: 238 TYPE: SA)

PAT	OF	NSG	G	Y	R	EWG	Y	R	NSL	Y	S	Y	M	CYC
1	T	0	40	1	4	1	15	4	1	7	3	7	76	DAY 0/8
2	T	0	40	1	4	1	15	4	1	7	3	6	76	NITE 8/0
3	T	0	45	1	4	1	15	4	1	7	3	7	81	EARLY AM PEA
4	T	0	45	1	4	1	15	4	1	7	3	7	81	EARLY AFT. 0
5	T	0	40	1	4	1	15	4	1	7	3	7	76	LATE PM / EV
6	T	0	45	1	4	1	15	4	1	7	3	7	81	SCHOOL IN 0/
7	T	0	40	1	4	1	15	4	1	7	3	7	76	SCH. OUT 0/8
8	T	0	45	1	4	1	15	4	1	7	3	7	81	SCHOOL OUT 0
10	T	0	40	1	4	1	15	4	1	7	3	7	76	PRE-AM PEAK
14	T	0	45	1	4	1	15	4	1	7	3	7	81	AFTERNOON 0
24	T	0	44	1	4	1	15	4	1	7	3	7	80	RE

4783

TIMING DATA FOR 4783 KROME AVE & SW 184 ST (SEC: 238 TYPE: SA)

PAT	OF	NSG	G	Y	R	EWG	Y	R	NSL	Y	S	Y	M	CYC
1	T	0	45	1	4	1	28	4	1	7	3	7	94	DAY 0/8
2	T	0	40	1	4	1	15	4	1	7	3	6	76	NITE 8/0
3	T	0	50	1	4	1	35	4	1	7	3	7	106	EARLY AM PEA
4	T	0	50	1	4	1	35	4	1	7	3	7	106	EARLY AFT. 0
5	T	0	45	1	4	1	28	4	1	7	3	7	94	LATE PM / EV
6	T	0	50	1	4	1	35	4	1	7	3	7	106	SCHOOL IN 0/
7	T	0	45	1	4	1	28	4	1	7	3	7	94	SCH. OUT 0/8
8	T	0	50	1	4	1	35	4	1	7	3	7	106	SCHOOL OUT 0
10	T	0	45	1	4	1	28	4	1	7	3	7	94	PRE-AM PEAK
14	T	0	50	1	4	1	35	4	1	7	3	7	106	AFTERNOON 0
24	T	0	44	1	4	1	15	4	1	7	3	7	80	RECALL TEST
MIN: 40 10 6														

4637

TIMING DATA FOR 4637 CORAL REEF & SW 137 AV (SEC: 281 TYPE: SA)

PAT	OF	NSW	F	Y	R	EW	M	Y	EW	F	G	Y	R	NSM	Y	S	Y	M	CYC
1	T	0	20	18	4	2	15	4	7	18	10	4	2	18	4	7	126	Off Peak	
2	M	0	18	18	4	2	27	4	7	18	30	4	2	22	4	160	PM Peak		
3	T	0	10	18	4	2	18	4	7	18	10	4	2	19	4	120	EVENING		
5	T	0	32	18	4	2	45	4	7	18	15	4	2	20	4	7	175	PRE-AM PEAK	
6	T	155	30	18	4	2	30	4	7	18	17	4	2	30	4	11	170	AM Peak	
9	T	0	10	18	4	2	15	4	7	18	1	4	2	15	4	7	104	Nite	
10	T	0	10	18	4	2	25	4	7	18	10	4	2	22	4	130	Midday		
11	T	0	14	18	4	2	22	4	7	18	12	4	2	19	4	130	Weekend		
16	M	0	16	18	4	2	55	4	7	18	16	4	2	20	4	11	170	PRE-AM PEAK	
19	T	0	10	18	4	2	14	4	7	18	1	4	2	12	4	100	EVENING		
20	T	0	22	18	4	2	30	4	7	18	25	4	2	20	4	160	PM Peak		
24	T	0	20	18	4	2	10	4	7	18	1	4	2	10	4	7	104	Recall Test	
MIN: 10 18 7 18 1 5																			

4764 0

TIMING DATA FOR 4764 CORAL REEF & SW 117 AV (SEC: 171 TYPE: SA)IDNIT
PAT OF EWW F Y R NSL Y NSW F G Y REWL Y S Y M CYC (SEC

7 M 5

1 T 59 11 12 4 1 8 3 5 17 1 4 1 15 3	85EARLY AM M2
3 T 67 18 12 4 1 6 3 5 17 1 4 1 15 3	90OFF PEAK M2
4 T 67 18 12 4 1 6 3 5 17 1 4 1 15 3	90EARLY NITE
5 T 69 11 12 4 1 12 3 5 17 1 4 1 26 3	100OFF PEAK M2
6 T 74 18 12 4 1 6 3 5 17 4 4 1 62 3	11 140AM PEAK M1 1
7 M 51 12 12 4 1 6 3 5 27 2 4 1 10 3	90PM PEAK M2 0
8 T 75 13 12 4 1 11 3 5 17 1 4 1 15 3	90OFF PEAK M1
9 T 32 12 12 4 1 6 3 5 17 1 4 1 6 3	7 75NITE 3/6
10 T 83 9 12 4 1 6 3 5 17 1 4 1 24 3	90EARLY & LATE
11 T 3 10 12 4 1 8 3 5 17 1 4 1 21 3	90WEEKEND
12 T 74 18 12 4 1 6 3 5 17 4 4 1 62 3	11 140AM PEAK M1 S
13 M 34 18 12 4 1 6 3 5 17 1 4 1 15 3	90ZOO OUT
14 M 77 14 12 4 1 6 3 5 17 3 4 1 27 3	11 100AM PEAK M1 1
15 T 75 13 12 4 1 11 3 5 17 1 4 1 15 3	90EARLY OFF PE
16 T 88 20 12 4 1 10 3 5 17 1 4 1 59 3	11 140PRE-AM PEAK
17 T 75 13 12 4 1 11 3 5 17 1 4 1 15 3	90LATE OFF PEA
18 T 75 13 12 4 1 11 3 5 17 1 4 1 15 3	90OFF PEAK SCH
20 T 78 27 12 4 1 12 3 5 22 1 4 1 15 3	110PM PEAK M2 0
23 T 52 17 12 4 1 6 3 5 17 1 4 1 6 3	7 80LATE NITE 11
24 T 0 20 12 4 1 6 3 5 17 1 4 1 15 3	7 92RECALL TEST
MIN: 7 12 6 27 1 5	

4396

TIMING DATA FOR 4396 CORAL REEF & SR 821 W (SEC: 171 TYPE: SA)

15 M 35 99 31 4

3972 4

3 T 54 52 1 4 1 14 1 4 2 8 3
4 T 54 52 1 4 1 14 1 4 2 8 3
5 T 54 62 1 4 1 12 1 4 2 10 3
6 T 25 95 1 4 1 15 3 4 2 12 3
7 M 53 35 1 4 1 20 10 4 2 10 3
8 T 37 53 1 4 1 14 1 4 2 7 3
9 T 33 30 1 4 1 9 1 4 2 5 3
10 T 73 53 1 4 1 13 1 4 2 8 3
11 T 6 45 1 4 1 21 1 4 2 8 3
12 T 25 95 1 4 1 15 3 4 2 12 3
13 M 51 55 1 4 1 11 1 4 2 8 3
14 M 49 61 1 4 1 13 1 4 2 10 3
15 T 37 53 1 4 1 14 1 4 2 7 3
16 T 41 95 1 4 1 15 3 4 2 12 3
17 T 45 53 1 4 1 14 1 4 2 7 3
18 T 37 53 1 4 1 14 1 4 2 7 3
20 T 42 66 1 4 1 18 1 4 2 10 3
23 T 52 42 1 4 1 12 1 4 2 5 3
24 T 0 30 1 4 1 24 1 4 2 7 3
MIN: 17 1 5

90OFF PEAK M2
90EARLY NITE
100OFF PEAK M2
140AM PEAK M1 1
90PM PEAK M2 0
90OFF PEAK M1
60NITE 3/6
90EARLY & LATE
90WEEKEND
140AM PEAK M1 S
90ZOO OUT
100AM PEAK M1 1
90EARLY OFF PE
140PRE-AM PEAK
90LATE OFF PEA
90OFF PEAK SCH
110PM PEAK M2 0
75LATE NITE 11
7 77RECALL TEST

$$(C \oplus 0 \oplus F) \dots 1$$
$$(C+0+E) = 125$$

Comments: (REV. 5/03)

ASSIGNABLE INPUTS

C1

AT'IBUTES

PHASES(S)

ASSIGNMENTS

A	B	C	D	E	F	PIN#	1	2	3	4	5	6	7	8	1	2	3	4	5	6	7	8	1	2	3	4	5	6	7	8
NOT3	MAX 2	PRETIM	X PERM 0	DIAL 2	DIMMING	0	39			4	5	7		1										1	2	3				
NOT4	SY DET 1	PLAN 1	X PERM 1	DIAL 3	EV A	1	40			4	5	7		1										1	2	3				
2 OR 4	SY DET 2	PLAN 2	X PERM 2	OFF 1	EV B	2	41			4	5	7		2										1	2	3				
3 OR 4	SY DET 3	PLAN 3	WK DAY	OFF 2	EV C	3	42			4	5	7		2										1	2	3				
4 OR 5	SY DET 4	PLAN 4	X CLOCK 54	OFF 3	EV D	4	43			4	5	7		3										1	2	3				
5 OR 5	SY DET 5	PLAN 5	ST TIME 82	FREE	RR 1	5	44			4	5	7		3										1	2	3				
6 OR 6	SY DET 6	PLAN 6	FL SENS 81	FLASH 70	RR 2	6	45			4	5	7		4										1	2	3				
7 OR 6	SY DET 7	PLAN 7	ENABLE 67	XPD OMT	SP 1	7	46			4	5	7		4										1	2	3				
8	SY DET 8	PLAN 8	ADVANC 69	NOT 1	SP 2	8	47			4	5	7		5										1	2	3				
9	MX INBT 79	PLAN 9	ALARM 65	NOT 2	EXT LAG	9	48			4	5	7		5										1	2	3				
A AND 4	FCRCE A 69	DELAY A	PH BNK 2	OR 1	AND 1	A	56			4	5	7											6	1	2	3				
B AND 4	FCRCE B 69	DELAY B	PH BNK 3	OR 1	AND 1	B	57			4	5	7											6	1	2	3				
C NAND 1	CNA 68	DELAY C	OL SET 2	OR 2	AND 2	C	58			4	5	7												1	2	3				
D NAND 1	HCLD 69	DELAY D	OL SET 3	OR 2	AND 2	D	59			4	5	7												1	2	3				
E NAND 2	VE CALL	DELAY E	DET ST 2	OR 3	AND 3	E	60			4	5	7												1	2	3				
F NAND 2	RECALL	DELAY F	DET ST 3	OR 3	AND 3	F	61			4	5	7												1	2	3				

(C+0+E)-126,

(E+COLUMN+ROW)

ENTER C1 (OR PSEUDO) PIN #

DETECTOR ASSIGNMENTS (C+0+E)-126, (E+CCL+ROW)

ASSIGNABLE OUTPUTS

COL4

COLUMN 5

COLUMN 6

COLUMN 7

C1

AT'IBUTES

PHASES(S)

ASSIGNMENTS

A	B	C	D	E	F	PIN#	1	2	3	4	5	6	7	8	1	2	3	4	5	6	7	8	1	2	3	4	5	6	7	8
0	FLHR 0	FREE	NOT 1	TOD 1	DIAL 2	0																								
1 SP EV 1	FLHR 1	PLAN 1	OR 1	TOD 2	DIAL 3	1																								
2 SP EV 2	F FLHR	PLAN 2	OR 2	TOD 3	OFF 1	2																								
3 SP EV 3	BUS G	PLAN 3	OR 3	TOD 4	OFF 2	3																								
4 SP EV 4	BUS Y	PLAN 4	AND 1	TOD 5	OFF 3	4																								
5 SP EV 5	BUS R	PLAN 5	AND 2	TOD 6	FREE	5																								
6 SP EV 6	BUS W	PLAN 6	AND 3	TOD 7	FLASH	6																								
7 SP EV 7	BUS F	PLAN 7	NOT 2	TOD 8	PREMPT	7																								
8 SP EV 8	NOT 3	PLAN 8	EV A	WARN 1		8	75	2						2											1	2	3			
9	NOT 4	PLAN 9	EV B	WARN 2		9	76	2						6											1	2	3			
A DET FAIL 83	OR 4		EV C	DELAY A		A	77	2						4											1	2	3			
B	OR 5		EV D	DELAY B		B	78	2						8											1	2	3			
C	OR 6		RR 1	DELAY C		C																								
D CNT CTL	AND 4		RR 2	DELAY D		D																								
E X DWALK	NAND 1		SP 1	DELAY E		E																								
F X WALK	NAND 2		SP 2	DELAY F		F																								

(C+0+E)=127,

(E+COLUMN+ROW)

ENTER C1 (OR PSEUDO) PIN #

DETECTOR ASSIGNMENTS (C+0+E)-126, (E+CCL+ROW)

(9+EVENT#) DAY OF WEEK

OFF S M T W T F S

TIME PLAN SET 1 2 3 4 5 6 7

(9+EVENT#) DAY OF WEEK

OFF S M T W T F S

TIME PLAN SET 1 2 3 4 5 6 7

Page 2 of 2

Location: KENDALL DR & KROME AV

Asset # 5726

(F+COL+ROW)	RR	BF	FIG	FIRE	(C+E+ROW)
PREEMPT MIN.	A	B	C		PREEMPT
A WALK					0 RR1 DLY
B DON'T WALK					1 RR1 CLR
C INITIAL					2 EVA DLY
					3 EVA CLR
(F+C+0)					4 EVB DLY
RED START					5 EVB CLR
					6 EVC DLY
(F+0+F)					7 EVC CLR
RED REVERT	2				8 EVD DLY
(C+0+F)-1					9 EVD CLR
					A RR2 DLY
					B RR2 CLR
					(C+0+F)-1

(C+0+9)-0.1

(C+0+9)=0.2

SIGNAL OPERATING PLAN

PHASE		INT	SIGNAL HEAD NUMBER										
			2	4	3/5	4/7	3/8	6	8				
Φ_{2+5} S/B OVERLAP KROME AV (ACTUATED)	R/W		G	R	G	R	R	R	R				
	TO	Φ_{2+6}	G	R	G	R	R	R	R				
	CLEAR												
Φ_{2+6} N/S KROME AV (RECALL)	R/W		G	R	G	R	R	G	R				
	TO	Φ_3	Y	R	Y	R	R	Y	R				
	CLEAR	Φ_4	Y	R	Y	R	R	Y	R				
Φ_3 E/B KENDALL DR (ACTUATED)	R/W		R	R	R	R	G	R	G				
	TO	Φ_4	R	R	R	R	Y	R	Y				
	CLEAR	Φ_{2+5}	R	R	R	R	Y	R	Y				
		Φ_{2+6}	R	R	R	R	Y	R	Y				
Φ_4 W/B KENDALL DR (ACTUATED) FLASH OP.	R/W		R	G	R	G	R	R	R				
	TO	Φ_{2+5}	R	Y	R	Y	R	R	R				
	CLEAR	Φ_{2+6}	R	Y	R	Y	R	R	R				

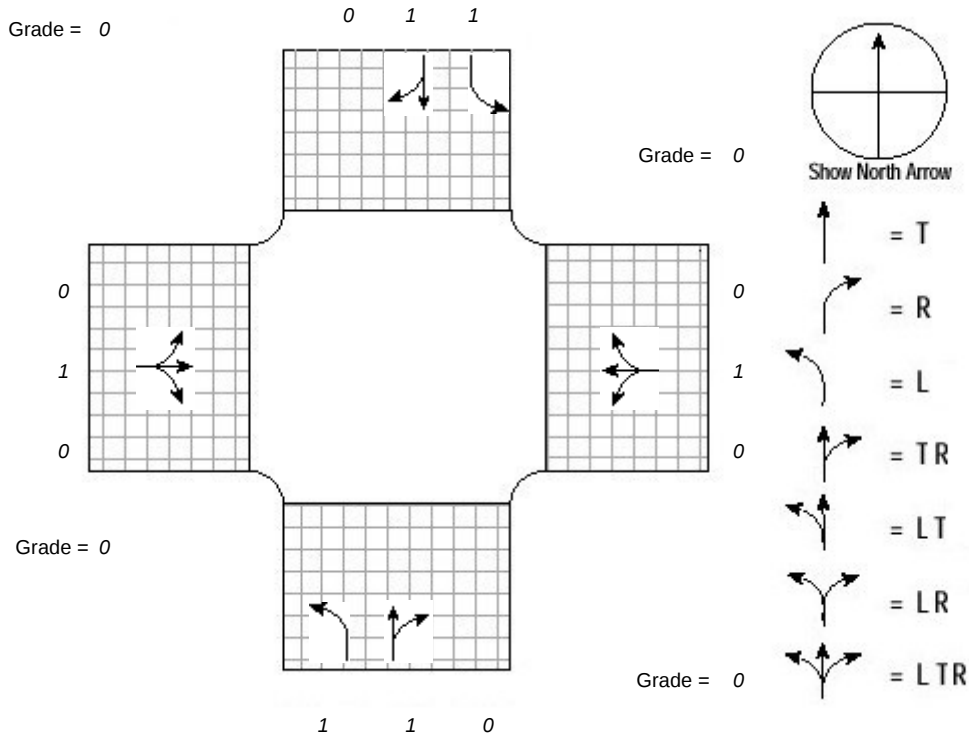
Drawn F. BADRAMPUR		Date 3/20/97	METROPOLITAN DADE COUNTY DEPARTMENT OF PUBLIC WORKS	
Check 		Date 3/21/97	REVISED 5/11/03 F.B.	ASSET NO: 5726
Division Engineer		Date	KENDALL DR & KROME AV	
Placed In Service Date: 4/4/97 By: C-TSC			Phasing Number 1	

Existing Conditions

FULL REPORT

General Information				Site Information	
Analyst	LSB			Intersection	SW 216 Street/SW 177 Avenue
Agency or Co.	Cathy Sweetapple & Associates			Area Type	All other areas
Date Performed	08/16/2008			Jurisdiction	Miami-Dade
Time Period	PM Peak Hour			Analysis Year	Existing

Intersection Geometry



Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume (vph)	49	86	75	45	88	58	29	582	48	72	678	56
% Heavy Veh	2	2	2	2	2	2	2	8	2	2	8	2
PHF	0.88	0.88	0.88	0.91	0.91	0.91	0.89	0.89	0.89	0.89	0.89	0.89
Actuated (P/A)	A	A	A	A	A	A	A	P	P	A	P	P
Startup Lost Time		2.0			2.0		2.0	2.0		2.0	2.0	
Extension of Effective Green		2.0			2.0		2.0	2.0		2.0	2.0	
Arrival Type		3			3		3	3		3	3	
Unit Extension		3.0			3.0		3.0	3.0		3.0	3.0	
Ped/Bike/RTOR Volume	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0			12.0		12.0	12.0		12.0	12.0	
Parking (Y or N)	N		N	N		N	N		N	N		N
Parking/Hour												
Bus Stops/Hour		0			0		0	0		0	0	

Pedestrian Timing			3.2		3.2		3.2		3.2	
	EW Perm	02	03	04	Excl. Left	NS Perm	07	08		
Timing	G = 15.0	G =	G =	G =	G = 7.0	G = 41.0	G =	G =		
	Y = 5	Y =	Y =	Y =	Y = 3	Y = 5	Y =	Y =		
Duration of Analysis (hrs) = 0.25					Cycle Length C = 76.0					

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	49	86	75	45	88	58	29	582	48	72	678	56
PHF	0.88	0.88	0.88	0.91	0.91	0.91	0.89	0.89	0.89	0.89	0.89	0.89
Adjusted Flow Rate	56	98	85	49	97	64	33	654	54	81	762	63
Lane Group		LTR			LTR		L	TR		L	TR	
Adjusted Flow Rate		239			210		33	708		81	825	
Proportion of LT or RT	0.234	--	0.356	0.233	--	0.305	0.000	--	0.076	0.000	--	0.076
Saturation Flow Rate												
Base Satflow		1900			1900		1900	1900		1900	1900	
Number of Lanes	0	1	0	0	1	0	1	1	0	1	1	0
f_w		1.000			1.000		1.000	1.000		1.000	1.000	
f_{HV}		0.980			0.980		0.980	0.930		0.980	0.930	
f_g		1.000			1.000		1.000	1.000		1.000	1.000	
f_p		1.000			1.000		1.000	1.000		1.000	1.000	
f_{bb}		1.000			1.000		1.000	1.000		1.000	1.000	
f_a		1.000			1.000		1.000	1.000		1.000	1.000	
f_{LU}		1.000			1.000		1.000	1.000		1.000	1.000	
f_{LT}		0.816	--		0.801	--	0.950	1.000	--	0.950	1.000	--
Secondary f_{LT}			--			--	0.116		--	0.192		--

f_{RT}	--	0.952		--	0.959		--	0.989		--	0.989	
f_{Lpb}		1.000	--		1.000	--	1.000	1.000	--	1.000	1.000	--
f_{Rpb}	--	1.000		--	1.000		--	1.000		--	1.000	
Adjusted Satflow		1448			1430		1770	1747		1770	1747	
Secondary Adjusted Satflow			--			--	216		--	357		--

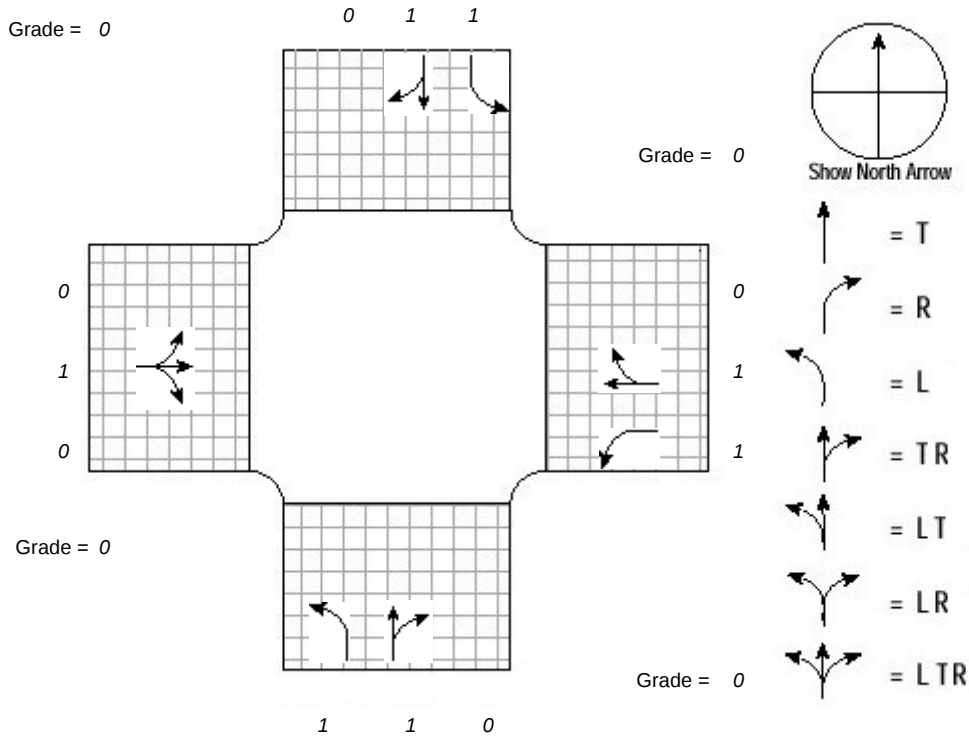
CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane Group		<i>LTR</i>			<i>LTR</i>		<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>	
Adjusted Flow Rate		239			210		33	708		81	825	
Satflow Rate		1448			1430		1770	1747		1770	1747	
Lost Time		2.0			2.0		2.0	2.0		2.0	2.0	
Green Ratio		0.20			0.20		0.67	0.54		0.67	0.54	
Lane Group Capacity		286			282		288	942		370	942	
v/c Ratio		0.84			0.74		0.11	0.75		0.22	0.88	
Flow Ratio		0.17			0.15		0.02	0.41		0.05	0.47	
Critical Lane Group		Y			N		N	N		Y	Y	
Sum Flow Ratios	0.68											
Lost Time/Cycle	15.00											
Critical v/c Ratio	0.85											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane Group		<i>LTR</i>			<i>LTR</i>		<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>	
Adjusted Flow Rate		239			210		33	708		81	825	
Lane Group Capacity		286			282		288	942		370	942	
v/c Ratio		0.84			0.74		0.11	0.75		0.22	0.88	
Green Ratio		0.20			0.20		0.67	0.54		0.67	0.54	
Uniform Delay d ₁		29.3			28.7		10.3	13.6		8.1	15.3	
Delay Factor k		0.37			0.30		0.11	0.50		0.11	0.50	
Incremental Delay d ₂		18.9			10.3		0.2	5.5		0.3	11.2	
PF Factor		1.000			1.000		1.000	1.000		1.000	1.000	

Control Delay		48.3			39.0		10.4	19.1		8.4	26.5	
Lane Group LOS		D			D		B	B		A	C	
Approach Delay	48.3			39.0			18.7			24.9		
Approach LOS	D			D			B			C		
Intersection Delay	26.8			Intersection LOS						C		

FULL REPORT

General Information				Site Information	
Analyst	LSB			Intersection	SW 200 Street/SW 177 Avenue
Agency or Co.	Cathy Sweetapple & Associates			Area Type	All other areas
Date Performed	08/16/2008			Jurisdiction	Miami-Dade
Time Period	PM Peak Hour			Analysis Year	Existing

Intersection Geometry



Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume (vph)	77	102	24	82	117	128	64	575	98	123	637	45
% Heavy Veh	2	2	2	2	2	2	2	8	2	2	8	2
PHF	0.83	0.83	0.83	0.91	0.91	0.91	0.93	0.93	0.93	0.93	0.93	0.93
Actuated (P/A)	A	A	A	A	A	A	A	P	P	A	P	P
Startup Lost Time		2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Extension of Effective Green		2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Arrival Type		3		3	3		3	3		3	3	
Unit Extension		3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Ped/Bike/RTOR Volume	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0		12.0	12.0		12.0	12.0		12.0	12.0	
Parking (Y or N)	N		N	N		N	N		N	N		N
Parking/Hour												
Bus Stops/Hour		0		0	0		0	0		0	0	

Pedestrian Timing			3.2		3.2		3.2		3.2	
	EW Perm	02	03	04	Excl. Left	NS Perm	07	08		
Timing	G = 18.0	G =	G =	G =	G = 7.0	G = 41.0	G =	G =		
	Y = 5	Y =	Y =	Y =	Y = 3	Y = 5	Y =	Y =		
Duration of Analysis (hrs) = 0.25					Cycle Length C = 79.0					

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	77	102	24	82	117	128	64	575	98	123	637	45
PHF	0.83	0.83	0.83	0.91	0.91	0.91	0.93	0.93	0.93	0.93	0.93	0.93
Adjusted Flow Rate	93	123	29	90	129	141	69	618	105	132	685	48
Lane Group		LTR		L	TR		L	TR		L	TR	
Adjusted Flow Rate		245		90	270		69	723		132	733	
Proportion of LT or RT	0.380	--	0.118	0.000	--	0.522	0.000	--	0.145	0.000	--	0.065
Saturation Flow Rate												
Base Satflow		1900		1900	1900		1900	1900		1900	1900	
Number of Lanes	0	1	0	1	1	0	1	1	0	1	1	0
f_w		1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_{HV}		0.980		0.980	0.980		0.980	0.933		0.980	0.929	
f_g		1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_p		1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_{bb}		1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_a		1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_{LU}		1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_{LT}		0.356	--	0.526	1.000	--	0.950	1.000	--	0.950	1.000	--
Secondary f_{LT}			--			--	0.157		--	0.164		--

f_{RT}	--	0.984		--	0.922		--	0.978		--	0.990	
f_{Lpb}		1.000	--	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--
f_{Rpb}	--	1.000		--	1.000		--	1.000		--	1.000	
Adjusted Satflow		653		980	1717		1770	1735		1770	1748	
Secondary Adjusted Satflow			--			--	293		--	306		--

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane Group		<i>LTR</i>		<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>	
Adjusted Flow Rate		245		90	270		69	723		132	733	
Satflow Rate		653		980	1717		1770	1735		1770	1748	
Lost Time		2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Green Ratio		0.23		0.23	0.23		0.65	0.52		0.65	0.52	
Lane Group Capacity		149		223	391		320	900		327	907	
v/c Ratio		1.64		0.40	0.69		0.22	0.80		0.40	0.81	
Flow Ratio		0.38		0.09	0.16		0.04	0.42		0.07	0.42	
Critical Lane Group		Y		N	N		N	N		Y	Y	
Sum Flow Ratios	0.87											
Lost Time/Cycle	15.00											
Critical v/c Ratio	1.07											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane Group		<i>LTR</i>		<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>	
Adjusted Flow Rate		245		90	270		69	723		132	733	
Lane Group Capacity		149		223	391		320	900		327	907	
v/c Ratio		1.64		0.40	0.69		0.22	0.80		0.40	0.81	
Green Ratio		0.23		0.23	0.23		0.65	0.52		0.65	0.52	
Uniform Delay d ₁		30.5		25.9	27.9		9.9	15.7		10.4	15.7	
Delay Factor k		0.50		0.11	0.26		0.11	0.50		0.11	0.50	
Incremental Delay d ₂		318.0		1.2	5.1		0.3	7.5		0.8	7.7	
PF Factor		1.000		1.000	1.000		1.000	1.000		1.000	1.000	

Control Delay		348.5		27.1	33.1		10.3	23.2		11.2	23.4	
Lane Group LOS		F		C	C		B	C		B	C	
Approach Delay	348.5			31.6			22.1			21.6		
Approach LOS	F			C			C			C		
Intersection Delay	58.8			Intersection LOS						E		

FULL REPORT

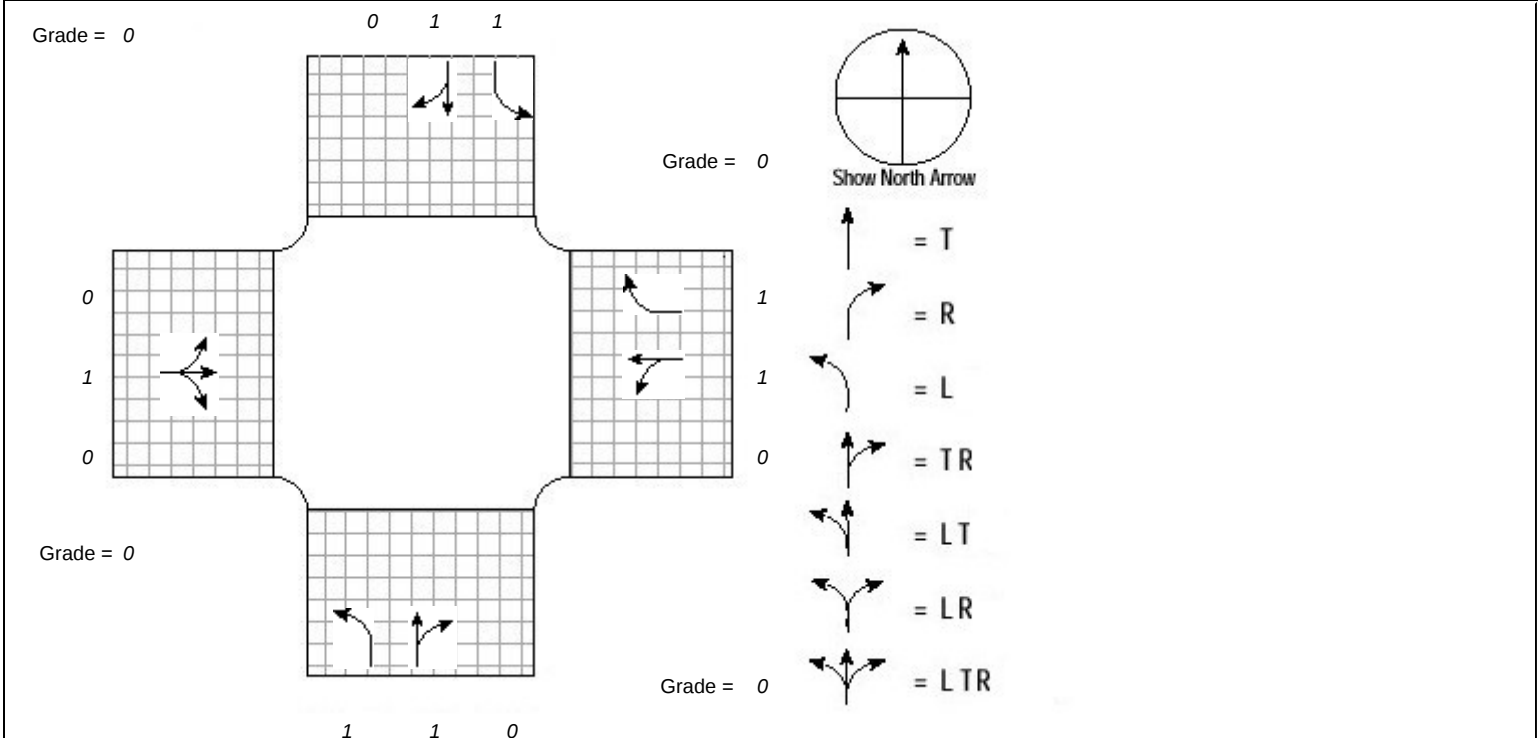
General Information	
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Analyst	LSB
Agency or Co.	Cathy Sweetapple & Associates
Date Performed	08/16/2008
Time Period	PM Peak Hour

Site Information

Intersection	SW 184 Street/Krome Avenue
Area Type	All other areas
Jurisdiction	Miami-Dade
Analysis Year	Existing

Intersection Geometry	
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Volume and Timing Input	
Volume	Timing
100	100
200	200
300	300
400	400
500	500
600	600
700	700
800	800
900	900
1000	1000

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume (vph)	62	30	7	212	25	99	3	609	196	156	688	26
% Heavy Veh	2	2	2	2	2	2	2	8	2	2	8	2
PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Actuated (P/A)	A	A	A	A	A	A	A	P	P	A	P	P
Startup Lost Time		2.0			2.0	2.0	2.0	2.0		2.0	2.0	
Extension of Effective Green		2.0			2.0	2.0	2.0	2.0		2.0	2.0	
Arrival Type		3			3	3	3	3		3	3	
Unit Extension		3.0			3.0	3.0	3.0	3.0		3.0	3.0	
Ped/Bike/RTOR Volume	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0			12.0	12.0	12.0	12.0		12.0	12.0	
Parking (Y or N)	N		N	N		N	N		N	N		N
Parking/Hour												
Bus Stops/Hour		0			0	0	0	0		0	0	

Pedestrian Timing			3.2		3.2		3.2		3.2	
	EW Perm	02	03	04	Excl. Left	NS Perm	07	08		
Timing	G = 28.0	G =	G =	G =	G = 7.0	G = 46.0	G =	G =		
	Y = 5	Y =	Y =	Y =	Y = 3	Y = 5	Y =	Y =		
Duration of Analysis (hrs) = 0.25					Cycle Length C = 94.0					

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	62	30	7	212	25	99	3	609	196	156	688	26
PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adjusted Flow Rate	69	33	8	236	28	110	3	677	218	173	764	29
Lane Group		LTR			LT	R	L	TR		L	TR	
Adjusted Flow Rate		110			264	110	3	895		173	793	
Proportion of LT or RT	0.627	--	0.073	0.894	--	0.000	0.000	--	0.244	0.000	--	0.037
Saturation Flow Rate												
Base Satflow		1900			1900	1900	1900	1900		1900	1900	
Number of Lanes	0	1	0	0	1	1	1	1	0	1	1	0
f_w		1.000			1.000	1.000	1.000	1.000		1.000	1.000	
f_{HV}		0.980			0.980	0.980	0.980	0.939		0.980	0.928	
f_g		1.000			1.000	1.000	1.000	1.000		1.000	1.000	
f_p		1.000			1.000	1.000	1.000	1.000		1.000	1.000	
f_{bb}		1.000			1.000	1.000	1.000	1.000		1.000	1.000	
f_a		1.000			1.000	1.000	1.000	1.000		1.000	1.000	
f_{LU}		1.000			1.000	1.000	1.000	1.000		1.000	1.000	
f_{LT}		0.513	--		0.691	--	0.950	1.000	--	0.950	1.000	--
Secondary f_{LT}			--			--	0.084		--	0.082		--

f_{RT}	--	0.990		--	1.000	0.850	--	0.963		--	0.995	
f_{Lpb}		1.000	--		1.000	--	1.000	1.000	--	1.000	1.000	--
f_{Rpb}	--	1.000		--	1.000	1.000	--	1.000		--	1.000	
Adjusted Satflow		945			1288	1583	1770	1718		1770	1753	
Secondary Adjusted Satflow			--			--	156		--	152		--

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane Group		<i>LTR</i>			<i>LT</i>	<i>R</i>	<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>	
Adjusted Flow Rate		<i>110</i>			<i>264</i>	<i>110</i>	<i>3</i>	<i>895</i>		<i>173</i>	<i>793</i>	
Satflow Rate		<i>945</i>			<i>1288</i>	<i>1583</i>	<i>1770</i>	<i>1718</i>		<i>1770</i>	<i>1753</i>	
Lost Time		<i>2.0</i>			<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	<i>2.0</i>		<i>2.0</i>	<i>2.0</i>	
Green Ratio		<i>0.30</i>			<i>0.30</i>	<i>0.30</i>	<i>0.60</i>	<i>0.49</i>		<i>0.60</i>	<i>0.49</i>	
Lane Group Capacity		<i>281</i>			<i>384</i>	<i>472</i>	<i>213</i>	<i>841</i>		<i>211</i>	<i>858</i>	
v/c Ratio		<i>0.39</i>			<i>0.69</i>	<i>0.23</i>	<i>0.01</i>	<i>1.06</i>		<i>0.82</i>	<i>0.92</i>	
Flow Ratio		<i>0.12</i>			<i>0.20</i>	<i>0.07</i>	<i>0.00</i>	<i>0.52</i>		<i>0.07</i>	<i>0.45</i>	
Critical Lane Group		<i>N</i>			<i>Y</i>	<i>N</i>	<i>N</i>	<i>Y</i>		<i>Y</i>	<i>N</i>	
Sum Flow Ratios	<i>0.80</i>											
Lost Time/Cycle	<i>15.00</i>											
Critical v/c Ratio	<i>0.95</i>											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane Group		<i>LTR</i>			<i>LT</i>	<i>R</i>	<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>	
Adjusted Flow Rate		<i>110</i>			<i>264</i>	<i>110</i>	<i>3</i>	<i>895</i>		<i>173</i>	<i>793</i>	
Lane Group Capacity		<i>281</i>			<i>384</i>	<i>472</i>	<i>213</i>	<i>841</i>		<i>211</i>	<i>858</i>	
v/c Ratio		<i>0.39</i>			<i>0.69</i>	<i>0.23</i>	<i>0.01</i>	<i>1.06</i>		<i>0.82</i>	<i>0.92</i>	
Green Ratio		<i>0.30</i>			<i>0.30</i>	<i>0.30</i>	<i>0.60</i>	<i>0.49</i>		<i>0.60</i>	<i>0.49</i>	
Uniform Delay d ₁		<i>26.2</i>			<i>29.1</i>	<i>24.9</i>	<i>15.3</i>	<i>24.0</i>		<i>19.7</i>	<i>22.4</i>	
Delay Factor k		<i>0.11</i>			<i>0.26</i>	<i>0.11</i>	<i>0.11</i>	<i>0.50</i>		<i>0.36</i>	<i>0.50</i>	
Incremental Delay d ₂		<i>0.9</i>			<i>5.1</i>	<i>0.3</i>	<i>0.0</i>	<i>49.6</i>		<i>22.0</i>	<i>17.1</i>	
PF Factor		<i>1.000</i>			<i>1.000</i>	<i>1.000</i>	<i>1.000</i>	<i>1.000</i>		<i>1.000</i>	<i>1.000</i>	

Control Delay		27.1			34.2	25.2	15.4	73.6		41.6	39.4	
Lane Group LOS		C			C	C	B	E		D	D	
Approach Delay	27.1			31.6			73.4			39.8		
Approach LOS	C			C			E			D		
Intersection Delay	50.7			Intersection LOS						D		

Pedestrian Timing			3.2		3.2		3.2		3.2	
	EB Only	WB Only	03	04	SB Only	NS Perm	07	08		
Timing	G = 15.0	G = 20.0	G =	G =	G = 7.0	G = 50.0	G =	G =		
	Y = 5	Y = 5	Y =	Y =	Y = 3	Y = 5	Y =	Y =		
Duration of Analysis (hrs) = 0.25							Cycle Length C = 110.0			

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	11	12	5	319	3	159	3	399	392	342	472	12
PHF	0.78	0.78	0.78	0.84	0.84	0.84	0.86	0.86	0.86	0.94	0.94	0.94
Adjusted Flow Rate	14	15	6	380	4	189	3	464	456	364	502	13
Lane Group		LTR			LT	R	L	T	R	L	T	R
Adjusted Flow Rate		35			384	189	3	464	456	364	502	13
Proportion of LT or RT	0.400	--	0.171	0.990	--	0.000	0.000	--	0.000	0.000	--	0.000
Saturation Flow Rate												
Base Satflow		1900			1900	1900	1900	1900	1900	1900	1900	1900
Number of Lanes	0	1	0	0	1	1	1	1	1	1	1	1
f_w		1.000			1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_{HV}		0.980			0.980	0.980	0.980	0.926	0.980	0.980	0.926	0.980
f_g		1.000			1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_p		1.000			1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_{bb}		1.000			1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_a		1.000			1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_{LU}		1.000			1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_{LT}		0.980	--		0.953	--	0.438	1.000	--	0.950	1.000	--
Secondary f_{LT}			--			--			--	0.301		--

f_{RT}	--	0.977		--	1.000	0.850	--	1.000	0.850	--	1.000	0.850
f_{Lpb}		1.000	--		1.000	--	1.000	1.000	--	1.000	1.000	--
f_{Rpb}	--	1.000		--	1.000	1.000	--	1.000	1.000	--	1.000	1.000
Adjusted Satflow		1784			1775	1583	815	1759	1583	1770	1759	1583
Secondary Adjusted Satflow			--			--			--	561		--

General Information

Capacity Analysis

Lane Group Capacity, Control Delay, and LOS Determination

Control Delay		42.1			156.7	47.2	16.4	23.3	24.8	65.5	16.4	11.5
Lane Group LOS		D			F	D	B	C	C	E	B	B
Approach Delay	42.1			120.6			24.0			36.7		
Approach LOS	D			F			C			D		
Intersection Delay	51.9			Intersection LOS						D		

ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	LSB	Intersection	SW 152 Street/SW 157 Avenue
Agency/Co.	Cathy Sweetapple & Associates	Jurisdiction	Miami-Dade
Date Performed	08/16/2008	Analysis Year	Existing
Analysis Time Period	PM Peak Hour		

Project ID *Parkland DRI*

East/West Street: *SW 152 Street*

North/South Street: *SW 157 Avenue*

Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
Movement	L	T	R	L	T	R
Volume (veh/h)	6	76	23	66	120	118
%Thrus Left Lane						

Approach	Northbound			Southbound		
Movement	L	T	R	L	T	R
Volume (veh/h)	39	85	69	192	170	29
%Thrus Left Lane	50			50		

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	<i>LTR</i>		<i>LTR</i>		<i>LT</i>	<i>TR</i>	<i>LT</i>	<i>TR</i>
PHF	<i>0.76</i>		<i>0.86</i>		<i>0.90</i>	<i>0.90</i>	<i>0.88</i>	<i>0.88</i>
Flow Rate (veh/h)	<i>137</i>		<i>352</i>		<i>89</i>	<i>123</i>	<i>314</i>	<i>128</i>
% Heavy Vehicles	<i>2</i>		<i>2</i>		<i>2</i>	<i>2</i>	<i>2</i>	<i>2</i>
No. Lanes	<i>1</i>		<i>1</i>		<i>2</i>		<i>2</i>	
Geometry Group	<i>2</i>		<i>2</i>		<i>5</i>		<i>5</i>	
Duration, T	<i>0.25</i>							

Saturation Headway Adjustment Worksheet

Prop. Left-Turns	<i>0.1</i>		<i>0.2</i>		<i>0.5</i>	<i>0.0</i>	<i>0.7</i>	<i>0.0</i>
Prop. Right-Turns	<i>0.2</i>		<i>0.4</i>		<i>0.0</i>	<i>0.6</i>	<i>0.0</i>	<i>0.3</i>
Prop. Heavy Vehicle	<i>0.0</i>		<i>0.0</i>		<i>0.0</i>	<i>0.0</i>	<i>0.0</i>	<i>0.0</i>
hLT-adj	<i>0.2</i>	<i>0.2</i>	<i>0.2</i>	<i>0.2</i>	<i>0.5</i>	<i>0.5</i>	<i>0.5</i>	<i>0.5</i>
hRT-adj	<i>-0.6</i>	<i>-0.6</i>	<i>-0.6</i>	<i>-0.6</i>	<i>-0.7</i>	<i>-0.7</i>	<i>-0.7</i>	<i>-0.7</i>
hHV-adj	<i>1.7</i>	<i>1.7</i>	<i>1.7</i>	<i>1.7</i>	<i>1.7</i>	<i>1.7</i>	<i>1.7</i>	<i>1.7</i>
hadj, computed	<i>-0.1</i>		<i>-0.2</i>		<i>0.3</i>	<i>-0.4</i>	<i>0.4</i>	<i>-0.1</i>

Departure Headway and Service Time

hd, initial value (s)	<i>3.20</i>		<i>3.20</i>		<i>3.20</i>	<i>3.20</i>	<i>3.20</i>	<i>3.20</i>
x, initial	<i>0.12</i>		<i>0.31</i>		<i>0.08</i>	<i>0.11</i>	<i>0.28</i>	<i>0.11</i>
hd, final value (s)	<i>6.45</i>		<i>5.88</i>		<i>7.16</i>	<i>6.47</i>	<i>6.85</i>	<i>6.31</i>
x, final value	<i>0.25</i>		<i>0.57</i>		<i>0.18</i>	<i>0.22</i>	<i>0.60</i>	<i>0.22</i>
Move-up time, m (s)	<i>2.0</i>		<i>2.0</i>		<i>2.3</i>		<i>2.3</i>	
Service Time, t _s (s)	<i>4.4</i>		<i>3.9</i>		<i>4.9</i>	<i>4.2</i>	<i>4.5</i>	<i>4.0</i>

Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	<i>387</i>		<i>583</i>		<i>339</i>	<i>373</i>	<i>509</i>	<i>378</i>
Delay (s/veh)	<i>11.53</i>		<i>16.52</i>		<i>11.40</i>	<i>11.00</i>	<i>19.18</i>	<i>10.83</i>
LOS	<i>B</i>		<i>C</i>		<i>B</i>	<i>B</i>	<i>C</i>	<i>B</i>
Approach: Delay (s/veh)	<i>11.53</i>		<i>16.52</i>		<i>11.17</i>		<i>16.76</i>	
LOS	<i>B</i>		<i>C</i>		<i>B</i>		<i>C</i>	
Intersection Delay (s/veh)	<i>15.02</i>							
Intersection LOS	<i>C</i>							

TWO-WAY STOP CONTROL SUMMARY

General Information								Site Information					
Analyst	LSB			Intersection	SW 152 Street/SW 147 Avenue								
Agency/Co.	Cathy Sweetapple & Associates			Jurisdiction	Miami-Dade								
Date Performed	08/16/2008			Analysis Year	Existing								
Analysis Time Period	PM Peak Hour												
Project Description								Parkland DRI					
East/West Street:				SW 152 Street				North/South Street:				SW 147 Avenue	
Intersection Orientation:				East-West				Study Period (hrs):				0.25	
Vehicle Volumes and Adjustments													
Major Street		Eastbound			Westbound								
Movement	1	2	3	4	5	6							
	L	T	R	L	T	R							
Volume (veh/h)		502	25	423	716								
Peak-Hour Factor, PHF	0.86	0.76	0.76	0.96	0.96	0.87							
Hourly Flow Rate, HFR (veh/h)	0	660	32	440	745	0							
Percent Heavy Vehicles	2	--	--	2	--	--							
Median Type	Undivided												
RT Channelized			0			0							
Lanes	0	2	0	1	2	0							
Configuration		T	TR	L	T								
Upstream Signal		0			0								
Minor Street		Northbound			Southbound								
Movement	7	8	9	10	11	12							
	L	T	R	L	T	R							
Volume (veh/h)	25		300										
Peak-Hour Factor, PHF	0.82	0.50	0.82	0.80	0.80	0.80							
Hourly Flow Rate, HFR (veh/h)	30	0	365	0	0	0							
Percent Heavy Vehicles	2	0	2	0	0	0							
Percent Grade (%)		0			0								
Flared Approach		N			N								
Storage		0			0								
RT Channelized			0			0							
Lanes	1	0	1	0	0	0							
Configuration	L		R										
Delay, Queue Length, and Level of Service													
Approach	Eastbound	Westbound	Northbound			Southbound							
Movement	1	4	7	8	9	10	11	12					
Lane Configuration		L	L		R								
v (veh/h)		440	30		365								
C (m) (veh/h)		899	30		650								
v/c		0.49	1.00		0.56								
95% queue length		2.74	3.35		3.50								
Control Delay (s/veh)		12.8	357.4		17.4								
LOS		B	F		C								
Approach Delay (s/veh)	--	--	43.2										
Approach LOS	--	--	E										

FULL REPORT	
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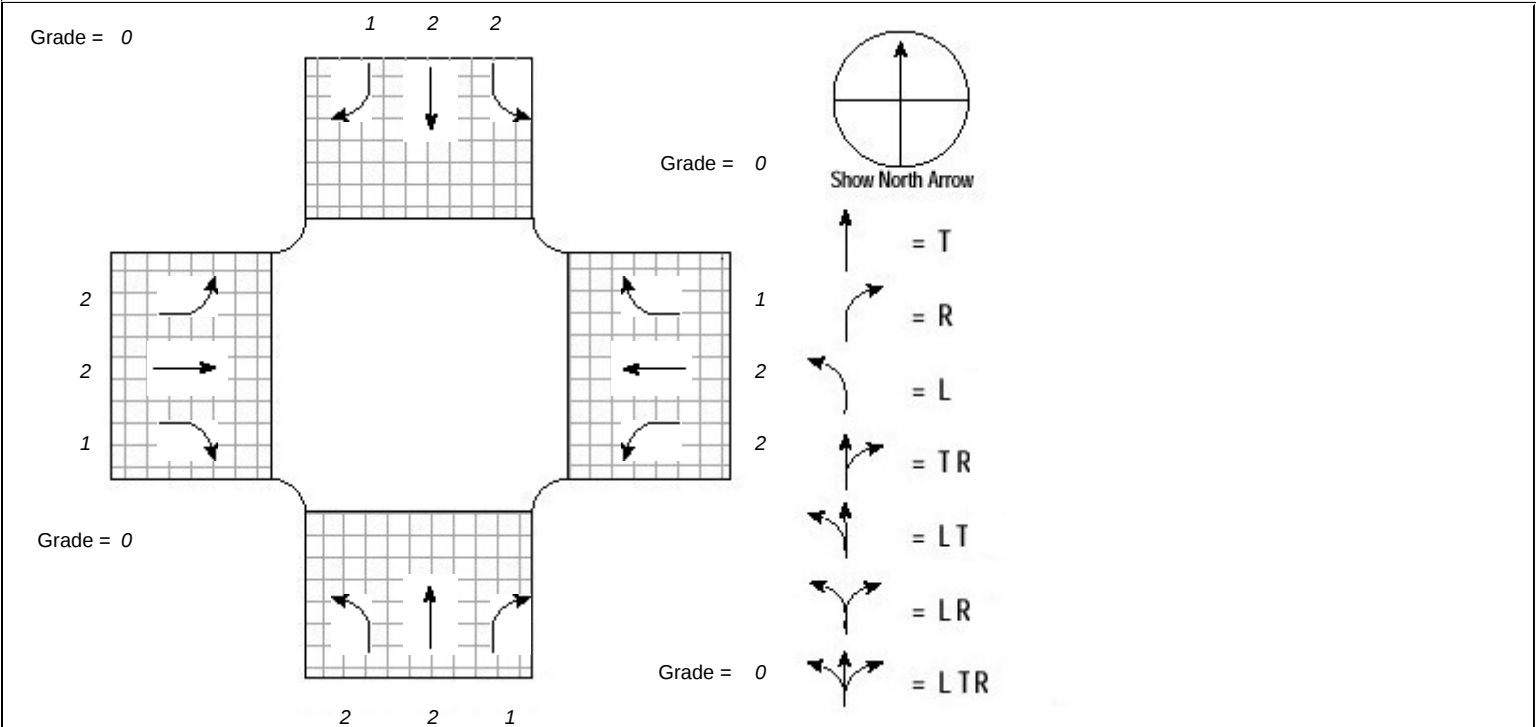
General Information	
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Analyst	LSB
Agency or Co.	Cathy Sweetapple & Associates
Date Performed	08/16/2008
Time Period	PM Peak Hour

Site Information

Intersection	SW 152 Street/SW 137 Avenue
Area Type	All other areas
Jurisdiction	Miami-Dade
Analysis Year	Existing

Intersection Geometry	
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Volume and Timing Input

[illegible]

Pedestrian Timing			3.2		3.2		3.2		3.2	
	Excl. Left	Thru & RT	03	04	Excl. Left	Thru & RT	07	08		
Timing	G = 30.0	G = 40.0	G =	G =	G = 20.0	G = 50.0	G =	G =		
	Y = 4	Y = 6	Y =	Y =	Y = 4	Y = 6	Y =	Y =		
Duration of Analysis (hrs) = 0.25					Cycle Length C = 160.0					

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	289	606	114	582	1086	555	294	732	269	532	1022	349
PHF	0.88	0.88	0.88	0.95	0.95	0.95	0.93	0.93	0.93	0.91	0.91	0.91
Adjusted Flow Rate	328	689	130	613	1143	584	316	787	289	585	1123	384
Lane Group	L	T	R	L	T	R	L	T	R	L	T	R
Adjusted Flow Rate	328	689	130	613	1143	584	316	787	289	585	1123	384
Proportion of LT or RT	0.000	--	0.000	0.000	--	0.000	0.000	--	0.000	0.000	--	0.000
Saturation Flow Rate												
Base Satflow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Number of Lanes	2	2	1	2	2	1	2	2	1	2	2	1
f_w	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_{HV}	0.980	0.980	0.980	0.980	0.980	0.980	0.980	0.980	0.980	0.980	0.980	0.980
f_g	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_p	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_{bb}	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_a	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_{LU}	0.971	0.952	1.000	0.971	0.952	1.000	0.971	0.952	1.000	0.971	0.952	1.000
f_{LT}	0.950	1.000	--	0.950	1.000	--	0.950	1.000	--	0.950	1.000	--
Secondary f_{LT}			--			--			--			--

f_{RT}	--	1.000	0.850	--	1.000	0.850	--	1.000	0.850	--	1.000	0.850
f_{Lpb}	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--
f_{Rpb}	--	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--	1.000	1.000
Adjusted Satflow	3437	3547	1583	3437	3547	1583	3437	3547	1583	3437	3547	1583
Secondary Adjusted Satflow			--			--			--			--

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>
Adjusted Flow Rate	328	689	130	613	1143	584	316	787	289	585	1123	384
Satflow Rate	3437	3547	1583	3437	3547	1583	3437	3547	1583	3437	3547	1583
Lost Time	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Green Ratio	0.19	0.25	0.25	0.19	0.25	0.25	0.13	0.31	0.31	0.13	0.31	0.31
Lane Group Capacity	644	887	396	644	887	396	430	1108	495	430	1108	495
v/c Ratio	0.51	0.78	0.33	0.95	1.29	1.47	0.73	0.71	0.58	1.36	1.01	0.78
Flow Ratio	0.10	0.19	0.08	0.18	0.32	0.37	0.09	0.22	0.18	0.17	0.32	0.24
Critical Lane Group	<i>N</i>	<i>N</i>	<i>N</i>	<i>Y</i>	<i>N</i>	<i>Y</i>	<i>N</i>	<i>N</i>	<i>N</i>	<i>Y</i>	<i>Y</i>	<i>N</i>
Sum Flow Ratios	1.03											
Lost Time/Cycle	20.00											
Critical v/c Ratio	1.18											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>
Adjusted Flow Rate	328	689	130	613	1143	584	316	787	289	585	1123	384
Lane Group Capacity	644	887	396	644	887	396	430	1108	495	430	1108	495
v/c Ratio	0.51	0.78	0.33	0.95	1.29	1.47	0.73	0.71	0.58	1.36	1.01	0.78
Green Ratio	0.19	0.25	0.25	0.19	0.25	0.25	0.13	0.31	0.31	0.13	0.31	0.31
Uniform Delay d ₁	58.4	55.8	49.0	64.3	60.0	60.0	67.4	48.6	46.3	70.0	55.0	49.9
Delay Factor k	0.12	0.33	0.11	0.46	0.50	0.50	0.29	0.50	0.50	0.50	0.50	0.50
	0.7	4.4	0.5	24.2	138.4	226.9	6.5	3.9	5.0	176.7	30.4	11.3

Incremental Delay d_2												
PF Factor	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Control Delay	59.1	60.3	49.5	88.4	198.4	286.9	73.9	52.5	51.2	246.7	85.4	61.2
Lane Group LOS	E	E	D	F	F	F	E	D	D	F	F	E
Approach Delay	58.7			191.7			57.1			126.1		
Approach LOS	E			F			E			F		
Intersection Delay	123.2			Intersection LOS						F		

Pedestrian Timing			3.2		3.2		3.2		3.2	
	Excl. Left	Thru & RT	03	04	Excl. Left	Thru & RT	07	08		
Timing	G = 15.0	G = 39.0	G =	G =	G = 12.0	G = 28.0	G =	G =		
	Y = 3	Y = 5	Y =	Y =	Y = 3	Y = 5	Y =	Y =		
Duration of Analysis (hrs) = 0.25							Cycle Length C = 110.0			

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	315	1264	208	37	1079	18	221	311	75	80	446	927
PHF	0.95	0.95	0.95	0.96	0.96	0.96	0.88	0.88	0.88	0.85	0.85	0.85
Adjusted Flow Rate	332	1331	219	39	1124	19	251	353	85	94	525	385
Lane Group	L	TR		L	T	R	L	T	R	L	T	R
Adjusted Flow Rate	332	1550		39	1124	19	251	353	85	94	525	385
Proportion of LT or RT	0.000	--	0.141	0.000	--	0.000	0.000	--	0.000	0.000	--	0.000
Saturation Flow Rate												
Base Satflow	1900	1900		1900	1900	1900	1900	1900	1900	1900	1900	1900
Number of Lanes	1	3	0	1	2	1	1	1	1	1	1	1
f_w	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_{HV}	0.980	0.980		0.980	0.980	0.980	0.980	0.980	0.980	0.980	0.980	0.980
f_g	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_p	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_{bb}	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_a	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_{LU}	1.000	0.908		1.000	0.952	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_{LT}	0.950	1.000	--	0.950	1.000	--	0.950	1.000	--	0.950	1.000	--
Secondary f_{LT}			--			--			--			--

f_{RT}	--	0.979		--	1.000	0.850	--	1.000	0.850	--	1.000	0.850
f_{Lpb}	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--
f_{Rpb}	--	1.000		--	1.000	1.000	--	1.000	1.000	--	1.000	1.000
Adjusted Satflow	1770	4967		1770	3547	1583	1770	1863	1583	1770	1863	1583
Secondary Adjusted Satflow			--			--			--			--

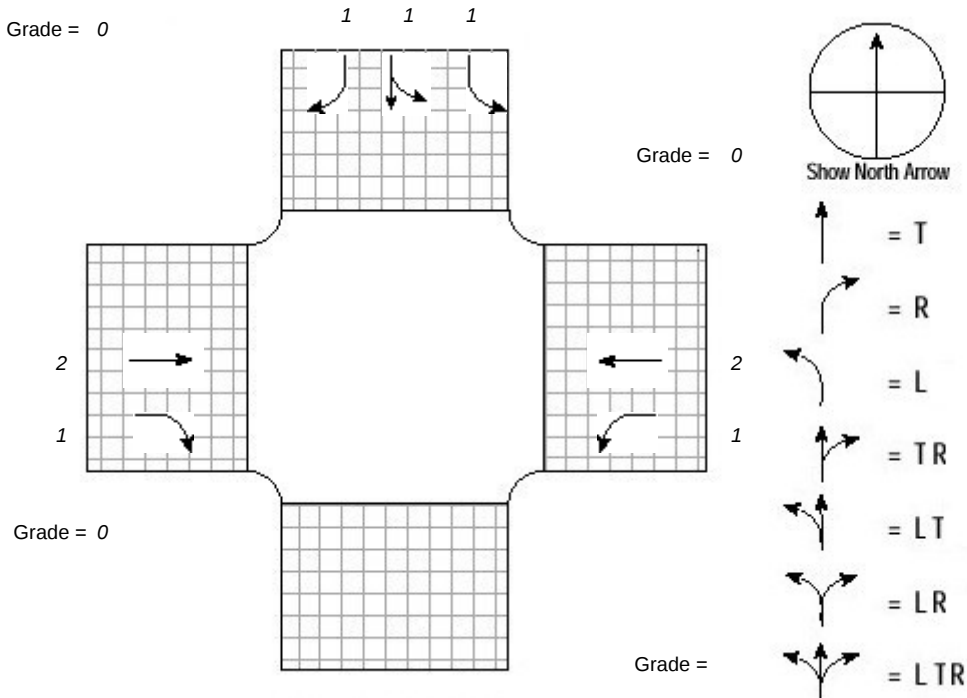
CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>TR</i>		<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>
Adjusted Flow Rate	332	1550		39	1124	19	251	353	85	94	525	385
Satflow Rate	1770	4967		1770	3547	1583	1770	1863	1583	1770	1863	1583
Lost Time	2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Green Ratio	0.14	0.35		0.14	0.35	0.35	0.11	0.25	0.25	0.11	0.25	0.25
Lane Group Capacity	241	1761		241	1258	561	193	474	403	193	474	403
v/c Ratio	1.38	0.88		0.16	0.89	0.03	1.30	0.74	0.21	0.49	1.11	0.96
Flow Ratio	0.19	0.31		0.02	0.32	0.01	0.14	0.19	0.05	0.05	0.28	0.24
Critical Lane Group	Y	N		N	Y	N	Y	N	N	N	Y	N
Sum Flow Ratios	0.93											
Lost Time/Cycle	16.00											
Critical v/c Ratio	1.09											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>TR</i>		<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>
Adjusted Flow Rate	332	1550		39	1124	19	251	353	85	94	525	385
Lane Group Capacity	241	1761		241	1258	561	193	474	403	193	474	403
v/c Ratio	1.38	0.88		0.16	0.89	0.03	1.30	0.74	0.21	0.49	1.11	0.96
Green Ratio	0.14	0.35		0.14	0.35	0.35	0.11	0.25	0.25	0.11	0.25	0.25
Uniform Delay d ₁	47.5	33.3		41.9	33.5	23.2	49.0	37.7	32.3	46.1	41.0	40.4
Delay Factor k	0.50	0.50		0.11	0.50	0.50	0.50	0.30	0.11	0.11	0.50	0.47
	193.8	6.7		0.3	9.9	0.1	167.8	6.3	0.3	1.9	74.0	33.4

Incremental Delay d_2												
PF Factor	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Control Delay	241.3	40.0		42.3	43.5	23.3	216.8	44.0	32.6	48.0	115.0	73.8
Lane Group LOS	F	D		D	D	C	F	D	C	D	F	E
Approach Delay	75.5			43.1			105.5			92.9		
Approach LOS	E			D			F			F		
Intersection Delay	75.5			Intersection LOS						E		

FULL REPORT

General Information				Site Information	
Analyst	LSB			Intersection	SW 152 Street/SR 821 W
Agency or Co.	Cathy Sweetapple & Associates			Area Type	All other areas
Date Performed	08/16/2008			Jurisdiction	Miami-Dade
Time Period	PM Peak Hour			Analysis Year	Existing

Intersection Geometry



Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume (vph)		1144	307	193	1016					388	99	123
% Heavy Veh		2	2	2	2					2	2	2
PHF		0.92	0.92	0.88	0.88					0.91	0.91	0.91
Actuated (P/A)		P	P	P	P					P	P	P
Startup Lost Time		2.0	2.0	2.0	2.0					2.0	2.0	2.0
Extension of Effective Green		2.0	2.0	2.0	2.0					2.0	2.0	2.0
Arrival Type		3	3	3	3					3	3	3
Unit Extension		3.0	3.0	3.0	3.0					3.0	3.0	3.0
Ped/Bike/RTOR Volume	0	0	0	0	0					0	0	0
Lane Width		12.0	12.0	12.0	12.0					12.0	12.0	12.0
Parking (Y or N)	N		N	N		N				N		N
Parking/Hour												
Bus Stops/Hour		0	0	0	0					0	0	0

Pedestrian Timing			3.2		3.2		3.2	
	WB Only	EW Perm	03	04	SB Only	06	07	08
Timing	G = 10.0	G = 67.0	G =	G =	G = 19.0	G =	G =	G =
	Y = 3	Y = 5	Y =	Y =	Y = 6	Y =	Y =	Y =
Duration of Analysis (hrs) = 0.25					Cycle Length C = 110.0			

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume		1144	307	193	1016					388	99	123
PHF		0.92	0.92	0.88	0.88					0.91	0.91	0.91
Adjusted Flow Rate		1243	334	219	1155					426	109	135
Lane Group		T	R	L	T					L	LT	R
Adjusted Flow Rate		1243	334	219	1155					426	109	135
Proportion of LT or RT	0.000	--	0.000	0.000	--	0.000		--		0.000	--	0.000
Saturation Flow Rate												
Base Satflow		1900	1900	1900	1900					1900	1900	1900
Number of Lanes		2	1	1	2					1	1	1
f_w		1.000	1.000	1.000	1.000					1.000	1.000	1.000
f_{HV}		0.980	0.980	0.980	0.980					0.980	0.980	0.980
f_g		1.000	1.000	1.000	1.000					1.000	1.000	1.000
f_p		1.000	1.000	1.000	1.000					1.000	1.000	1.000
f_{bb}		1.000	1.000	1.000	1.000					1.000	1.000	1.000
f_a		1.000	1.000	1.000	1.000					1.000	1.000	1.000
f_{LU}		0.952	1.000	1.000	0.952					1.000	1.000	1.000
f_{LT}		1.000	--	0.950	1.000	--			--	0.950	1.000	--
Secondary f_{LT}			--	0.147	0.780	--			--			--

f_{RT}	--	1.000	0.850	--	1.000		--			--	1.000	0.850
f_{Lpb}		1.000	--	1.000	1.000	--			--	1.000	1.000	--
f_{Rpb}	--	1.000	1.000	--	1.000		--			--	1.000	1.000
Adjusted Satflow		3547	1583	1770	3547					1770	1863	1583
Secondary Adjusted Satflow			--	273	2767	--			--			--

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane Group		<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>					<i>L</i>	<i>LT</i>	<i>R</i>
Adjusted Flow Rate		<i>1243</i>	<i>334</i>	<i>219</i>	<i>1155</i>					<i>426</i>	<i>109</i>	<i>135</i>
Satflow Rate		<i>3547</i>	<i>1583</i>	<i>1770</i>	<i>3547</i>					<i>1770</i>	<i>1863</i>	<i>1583</i>
Lost Time		<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	<i>2.0</i>					<i>2.0</i>	<i>2.0</i>	<i>2.0</i>
Green Ratio		<i>0.61</i>	<i>0.61</i>	<i>0.73</i>	<i>0.73</i>					<i>0.17</i>	<i>0.17</i>	<i>0.17</i>
Lane Group Capacity		<i>2160</i>	<i>964</i>	<i>335</i>	<i>2580</i>					<i>306</i>	<i>322</i>	<i>273</i>
v/c Ratio		<i>0.58</i>	<i>0.35</i>	<i>0.65</i>	<i>0.45</i>					<i>1.39</i>	<i>0.34</i>	<i>0.49</i>
Flow Ratio		<i>0.35</i>	<i>0.21</i>	<i>0.09</i>	<i>0.33</i>					<i>0.24</i>	<i>0.06</i>	<i>0.09</i>
Critical Lane Group		<i>Y</i>	<i>N</i>	<i>Y</i>	<i>N</i>					<i>Y</i>	<i>N</i>	<i>N</i>
Sum Flow Ratios	<i>0.68</i>											
Lost Time/Cycle	<i>16.00</i>											
Critical v/c Ratio	<i>0.80</i>											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane Group		<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>					<i>L</i>	<i>LT</i>	<i>R</i>
Adjusted Flow Rate		<i>1243</i>	<i>334</i>	<i>219</i>	<i>1155</i>					<i>426</i>	<i>109</i>	<i>135</i>
Lane Group Capacity		<i>2160</i>	<i>964</i>	<i>335</i>	<i>2580</i>					<i>306</i>	<i>322</i>	<i>273</i>
v/c Ratio		<i>0.58</i>	<i>0.35</i>	<i>0.65</i>	<i>0.45</i>					<i>1.39</i>	<i>0.34</i>	<i>0.49</i>
Green Ratio		<i>0.61</i>	<i>0.61</i>	<i>0.73</i>	<i>0.73</i>					<i>0.17</i>	<i>0.17</i>	<i>0.17</i>
Uniform Delay d ₁		<i>12.9</i>	<i>10.7</i>	<i>10.4</i>	<i>6.1</i>					<i>45.5</i>	<i>40.0</i>	<i>41.2</i>
Delay Factor k		<i>0.50</i>	<i>0.50</i>	<i>0.50</i>	<i>0.50</i>					<i>0.50</i>	<i>0.50</i>	<i>0.50</i>
		<i>1.1</i>	<i>1.0</i>	<i>9.6</i>	<i>0.6</i>					<i>195.3</i>	<i>2.8</i>	<i>6.3</i>

Incremental Delay d_2												
PF Factor		1.000	1.000	1.000	1.000					1.000	1.000	1.000
Control Delay		14.1	11.6	19.9	6.6					240.8	42.8	47.4
Lane Group LOS		B	B	B	A					F	D	D
Approach Delay	13.5			8.7						169.7		
Approach LOS	B			A						F		
Intersection Delay	40.6			Intersection LOS						D		

ALL-WAY STOP CONTROL ANALYSIS

General Information		Site Information	
Analyst	LSB	Intersection	SW 184 Street/SW 147 Avenue
Agency/Co.	Cathy Sweetapple & Associates	Jurisdiction	Miami-Dade
Date Performed	08/16/2008	Analysis Year	Existing
Analysis Time Period	PM Peak Hour		

Project ID *Parkland DRI*

East/West Street: SW 184 Street

North/South Street: SW 147 Avenue

Volume Adjustments and Site Characteristics

Approach	Eastbound			Westbound		
Movement	L	T	R	L	T	R
Volume (veh/h)	43	285	80	19	111	65
%Thrus Left Lane						

Approach	Northbound			Southbound		
Movement	L	T	R	L	T	R
Volume (veh/h)	79	244	49	77	199	27
%Thrus Left Lane						

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Configuration	LTR		LTR		LTR		LTR	
PHF	0.94		0.83		0.93		0.85	
Flow Rate (veh/h)	433		233		398		355	
% Heavy Vehicles	2		2		2		2	
No. Lanes	1		1		1		1	
Geometry Group	1		1		1		1	
Duration, T	0.25							

Saturation Headway Adjustment Worksheet

Prop. Left-Turns	0.1		0.1		0.2		0.3	
Prop. Right-Turns	0.2		0.3		0.1		0.1	
Prop. Heavy Vehicle	0.0		0.0		0.0		0.0	
hLT-adj	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2
hRT-adj	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6	-0.6
hHV-adj	1.7	1.7	1.7	1.7	1.7	1.7	1.7	1.7
hadj, computed	-0.1		-0.1		-0.0		0.0	

Departure Headway and Service Time

hd, initial value (s)	3.20		3.20		3.20		3.20	
x, initial	0.38		0.21		0.35		0.32	
hd, final value (s)	8.06		8.89		8.24		8.44	
x, final value	0.97		0.58		0.91		0.83	
Move-up time, m (s)	2.0		2.0		2.0		2.0	
Service Time, t _s (s)	6.1		6.9		6.2		6.4	

Capacity and Level of Service

	Eastbound		Westbound		Northbound		Southbound	
	L1	L2	L1	L2	L1	L2	L1	L2
Capacity (veh/h)	443		376		427		412	
Delay (s/veh)	63.93		23.28		52.68		41.39	
LOS	F		C		F		E	
Approach: Delay (s/veh)	63.93		23.28		52.68		41.39	
LOS	F		C		F		E	
Intersection Delay (s/veh)	48.46							
Intersection LOS	E							

TWO-WAY STOP CONTROL SUMMARY

General Information								Site Information			
Analyst <i>LSB</i>				Intersection <i>SW 184 Street/SW 157 Avenue</i>							
Agency/Co. <i>Cathy Sweetapple & Associates</i>				Jurisdiction <i>Miami-Dade</i>							
Date Performed <i>08/16/2008</i>				Analysis Year <i>Existing</i>							
Analysis Time Period <i>PM Peak Hour</i>											
Project Description <i>Parkland DRI</i>											
East/West Street: <i>SW 184 Street</i>				North/South Street: <i>SW 157 Avenue</i>							
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>							
Vehicle Volumes and Adjustments											
Major Street	Eastbound			Westbound							
Movement	1	2	3	4	5	6					
	L	T	R	L	T	R					
Volume (veh/h)	111	332	0	0	157	74					
Peak-Hour Factor, PHF	0.86	0.86	0.86	0.87	0.87	0.87					
Hourly Flow Rate, HFR (veh/h)	129	386	0	0	180	85					
Percent Heavy Vehicles	2	--	--	2	--	--					
Median Type	Undivided										
RT Channelized			0			0					
Lanes	0	1	0	0	1	0					
Configuration	LTR			LTR							
Upstream Signal		0			0						
Minor Street	Northbound			Southbound							
Movement	7	8	9	10	11	12					
	L	T	R	L	T	R					
Volume (veh/h)	0	1	1	85	0	154					
Peak-Hour Factor, PHF	0.50	0.50	0.50	0.80	0.80	0.80					
Hourly Flow Rate, HFR (veh/h)	0	2	2	106	0	192					
Percent Heavy Vehicles	2	2	2	2	2	0					
Percent Grade (%)	0			0							
Flared Approach		N			N						
Storage		0			0						
RT Channelized			0			0					
Lanes	0	1	0	0	1	0					
Configuration		LTR			LTR						
Delay, Queue Length, and Level of Service											
Approach	Eastbound	Westbound	Northbound			Southbound					
Movement	1	4	7	8	9	10	11	12			
Lane Configuration	LTR	LTR		LTR			LTR				
v (veh/h)	129	0		4			298				
C (m) (veh/h)	1299	1172		352			446				
v/c	0.10	0.00		0.01			0.67				
95% queue length	0.33	0.00		0.03			4.80				
Control Delay (s/veh)	8.1	8.1		15.3			27.9				
LOS	A	A		C			D				
Approach Delay (s/veh)	--	--	15.3			27.9					
Approach LOS	--	--	C			D					

Future without Project

FULL REPORT	
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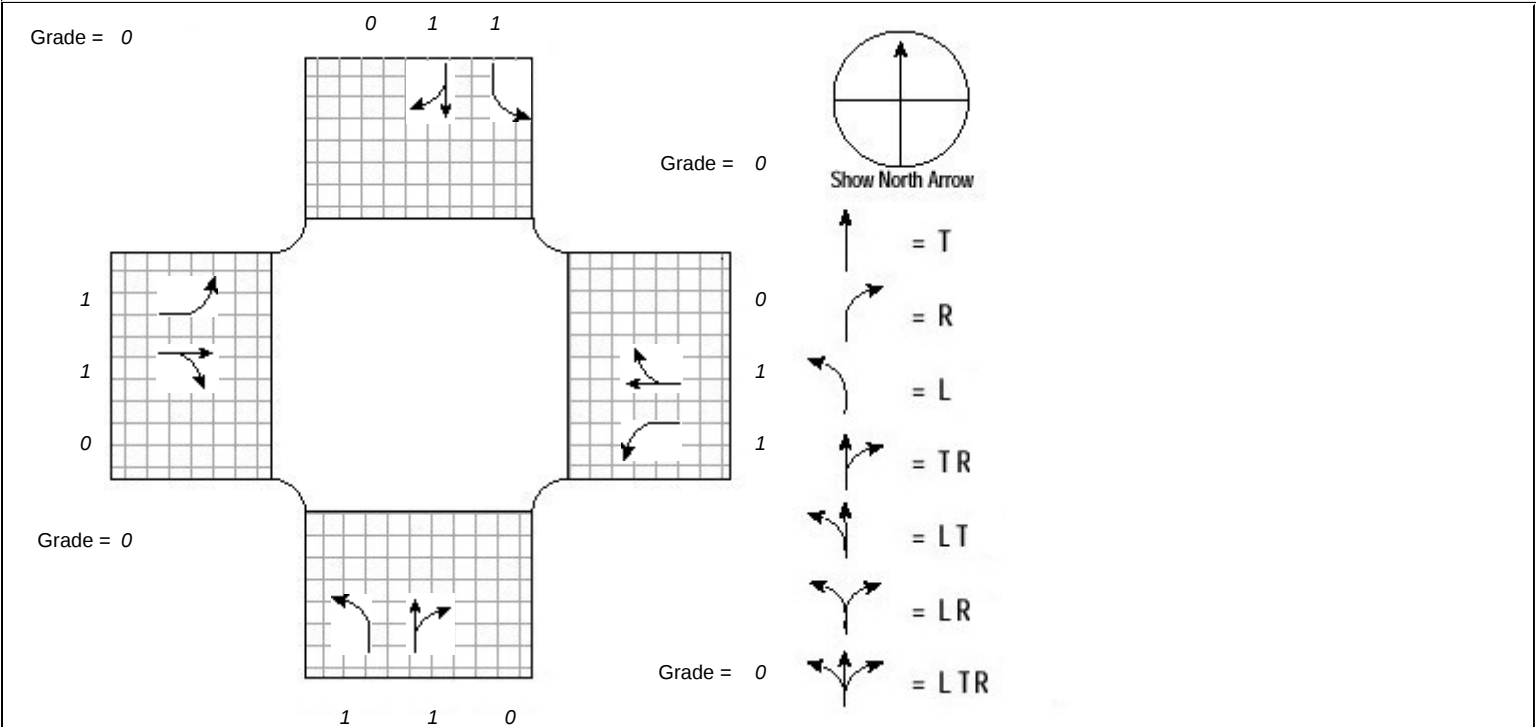
General Information	
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Analyst	LSB
Agency or Co.	Cathy Sweetapple & Associates
Date Performed	08/16/2008
Time Period	PM Peak Hour

Site Information

Intersection	SW 216 Street/SW 177 Avenue
Area Type	All other areas
Jurisdiction	Miami-Dade
Analysis Year	Future Without Project

Intersection Geometry



Volume and Timing Input					

[illegible]

Pedestrian Timing			3.2		3.2		3.2		3.2	
	EW Perm	02	03	04	Excl. Left	NS Perm	07	08		
Timing	G = 15.0	G =	G =	G =	G = 7.0	G = 41.0	G =	G =		
	Y = 5	Y =	Y =	Y =	Y = 3	Y = 5	Y =	Y =		
Duration of Analysis (hrs) = 0.25					Cycle Length C = 76.0					

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	52	93	81	48	95	63	31	683	51	78	813	61
PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adjusted Flow Rate	55	98	85	51	100	66	33	719	54	82	856	64
Lane Group	L	TR		L	TR		L	TR		L	TR	
Adjusted Flow Rate	55	183		51	166		33	773		82	920	
Proportion of LT or RT	0.000	--	0.464	0.000	--	0.398	0.000	--	0.070	0.000	--	0.070
Saturation Flow Rate												
Base Satflow	1900	1900		1900	1900		1900	1900		1900	1900	
Number of Lanes	1	1	0	1	1	0	1	1	0	1	1	0
f_w	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_{HV}	0.980	0.980		0.980	0.980		0.980	0.930		0.980	0.930	
f_g	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_p	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_{bb}	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_a	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_{LU}	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_{LT}	0.600	1.000	--	0.561	1.000	--	0.950	1.000	--	0.950	1.000	--
Secondary f_{LT}			--			--	0.091		--	0.150		--

f_{RT}	--	0.930		--	0.940		--	0.990		--	0.990	
f_{Lpb}	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--
f_{Rpb}	--	1.000		--	1.000		--	1.000		--	1.000	
Adjusted Satflow	1118	1733		1044	1752		1770	1748		1770	1748	
Secondary Adjusted Satflow			--			--	169		--	279		--

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>	
Adjusted Flow Rate	55	183		51	166		33	773		82	920	
Satflow Rate	1118	1733		1044	1752		1770	1748		1770	1748	
Lost Time	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Green Ratio	0.20	0.20		0.20	0.20		0.67	0.54		0.67	0.54	
Lane Group Capacity	221	342		206	346		261	943		325	943	
v/c Ratio	0.25	0.54		0.25	0.48		0.13	0.82		0.25	0.98	
Flow Ratio	0.05	0.11		0.05	0.09		0.02	0.44		0.05	0.53	
Critical Lane Group	<i>N</i>	<i>Y</i>		<i>N</i>	<i>N</i>		<i>N</i>	<i>N</i>		<i>Y</i>	<i>Y</i>	
Sum Flow Ratios	0.68											
Lost Time/Cycle	15.00											
Critical v/c Ratio	0.85											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>	
Adjusted Flow Rate	55	183		51	166		33	773		82	920	
Lane Group Capacity	221	342		206	346		261	943		325	943	
v/c Ratio	0.25	0.54		0.25	0.48		0.13	0.82		0.25	0.98	
Green Ratio	0.20	0.20		0.20	0.20		0.67	0.54		0.67	0.54	
Uniform Delay d ₁	25.7	27.4		25.7	27.0		13.3	14.4		9.5	17.0	
Delay Factor k	0.11	0.14		0.11	0.11		0.11	0.50		0.11	0.50	
Incremental Delay d ₂	0.6	1.7		0.6	1.1		0.2	7.9		0.4	24.0	
PF Factor	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	

Control Delay	26.3	29.0		26.4	28.1		13.5	22.4		9.9	41.0	
Lane Group LOS	C	C		C	C		B	C		A	D	
Approach Delay	28.4			27.7			22.0			38.4		
Approach LOS	C			C			C			D		
Intersection Delay	30.5			Intersection LOS						C		

FULL REPORT	
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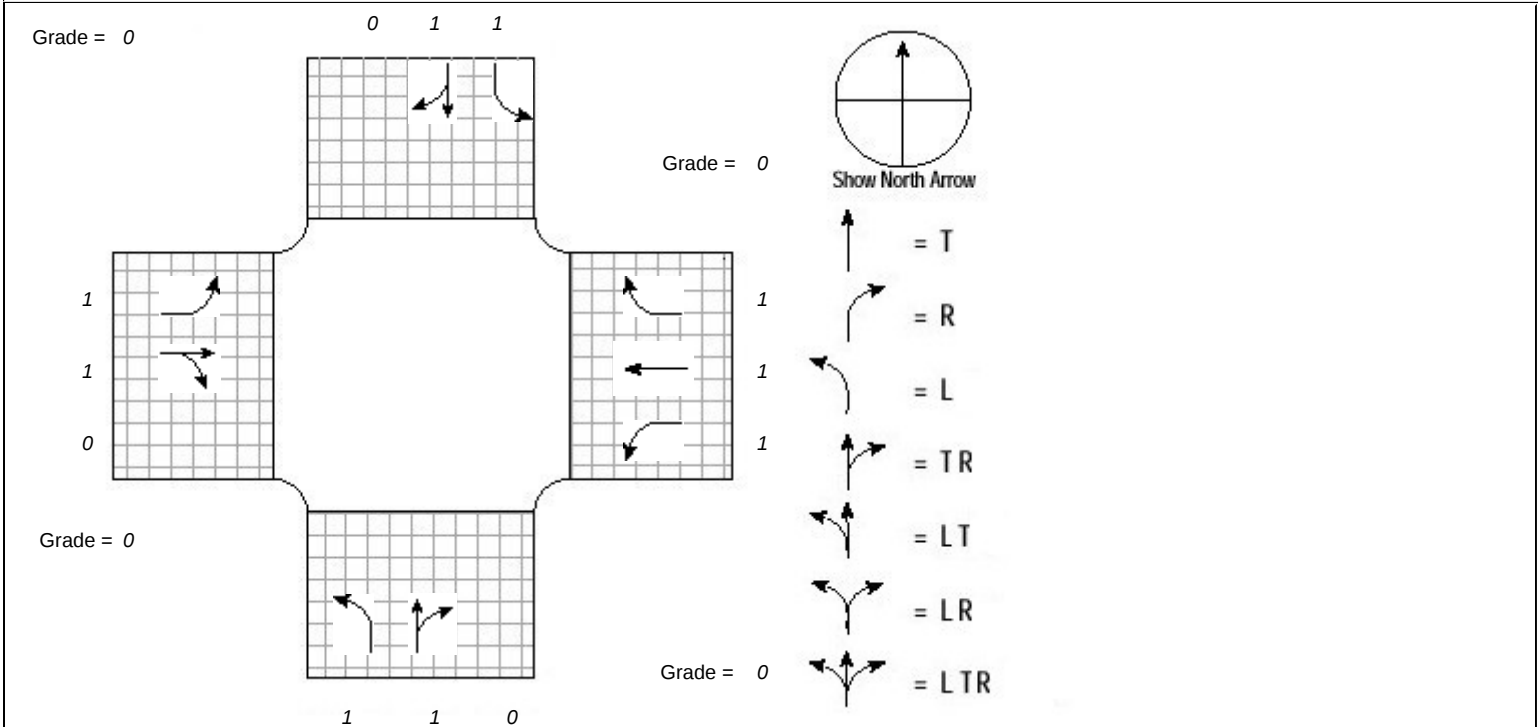
General Information	
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Analyst	LSB
Agency or Co.	Cathy Sweetapple & Associates
Date Performed	08/16/2008
Time Period	PM Peak Hour

Site Information

Intersection	<i>SW 200 Street/SW 177 Avenue</i>
Area Type	<i>All other areas</i>
Jurisdiction	<i>Miami-Dade</i>
Analysis Year	<i>Future Without Project</i>

Intersection Geometry



Volume and Timing Input

[illegible]

Pedestrian Timing			3.2		3.2		3.2		3.2	
	EW Perm	02	03	04	Excl. Left	NS Perm	07	08		
Timing	G = 18.0	G =	G =	G =	G = 7.0	G = 41.0	G =	G =		
	Y = 5	Y =	Y =	Y =	Y = 3	Y = 5	Y =	Y =		
Duration of Analysis (hrs) = 0.25					Cycle Length C = 79.0					

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	83	110	26	89	126	138	69	675	106	132	768	48
PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adjusted Flow Rate	87	116	27	94	133	145	73	711	112	139	808	51
Lane Group	L	TR		L	T	R	L	TR		L	TR	
Adjusted Flow Rate	87	143		94	133	145	73	823		139	859	
Proportion of LT or RT	0.000	--	0.189	0.000	--	0.000	0.000	--	0.136	0.000	--	0.059
Saturation Flow Rate												
Base Satflow	1900	1900		1900	1900	1900	1900	1900		1900	1900	
Number of Lanes	1	1	0	1	1	1	1	1	0	1	1	0
f_w	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
f_{HV}	0.980	0.980		0.980	0.980	0.980	0.980	0.933		0.980	0.929	
f_g	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
f_p	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
f_{bb}	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
f_a	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
f_{LU}	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
f_{LT}	0.672	1.000	--	0.656	1.000	--	0.950	1.000	--	0.950	1.000	--
Secondary f_{LT}			--			--	0.091		--	0.097		--

f_{RT}	--	0.972		--	1.000	0.850	--	0.980		--	0.991	
f_{Lpb}	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--
f_{Rpb}	--	1.000		--	1.000	1.000	--	1.000		--	1.000	
Adjusted Satflow	1252	1810		1222	1863	1583	1770	1736		1770	1749	
Secondary Adjusted Satflow			--			--	169		--	181		--

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>TR</i>		<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>	
Adjusted Flow Rate	<i>87</i>	<i>143</i>		<i>94</i>	<i>133</i>	<i>145</i>	<i>73</i>	<i>823</i>		<i>139</i>	<i>859</i>	
Satflow Rate	<i>1252</i>	<i>1810</i>		<i>1222</i>	<i>1863</i>	<i>1583</i>	<i>1770</i>	<i>1736</i>		<i>1770</i>	<i>1749</i>	
Lost Time	<i>2.0</i>	<i>2.0</i>		<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	<i>2.0</i>		<i>2.0</i>	<i>2.0</i>	
Green Ratio	<i>0.23</i>	<i>0.23</i>		<i>0.23</i>	<i>0.23</i>	<i>0.23</i>	<i>0.65</i>	<i>0.52</i>		<i>0.65</i>	<i>0.52</i>	
Lane Group Capacity	<i>285</i>	<i>412</i>		<i>278</i>	<i>424</i>	<i>361</i>	<i>251</i>	<i>901</i>		<i>258</i>	<i>908</i>	
v/c Ratio	<i>0.31</i>	<i>0.35</i>		<i>0.34</i>	<i>0.31</i>	<i>0.40</i>	<i>0.29</i>	<i>0.91</i>		<i>0.54</i>	<i>0.95</i>	
Flow Ratio	<i>0.07</i>	<i>0.08</i>		<i>0.08</i>	<i>0.07</i>	<i>0.09</i>	<i>0.04</i>	<i>0.47</i>		<i>0.08</i>	<i>0.49</i>	
Critical Lane Group	<i>N</i>	<i>N</i>		<i>N</i>	<i>N</i>	<i>Y</i>	<i>N</i>	<i>N</i>		<i>Y</i>	<i>Y</i>	
Sum Flow Ratios	<i>0.66</i>											
Lost Time/Cycle	<i>15.00</i>											
Critical v/c Ratio	<i>0.82</i>											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>TR</i>		<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>	
Adjusted Flow Rate	<i>87</i>	<i>143</i>		<i>94</i>	<i>133</i>	<i>145</i>	<i>73</i>	<i>823</i>		<i>139</i>	<i>859</i>	
Lane Group Capacity	<i>285</i>	<i>412</i>		<i>278</i>	<i>424</i>	<i>361</i>	<i>251</i>	<i>901</i>		<i>258</i>	<i>908</i>	
v/c Ratio	<i>0.31</i>	<i>0.35</i>		<i>0.34</i>	<i>0.31</i>	<i>0.40</i>	<i>0.29</i>	<i>0.91</i>		<i>0.54</i>	<i>0.95</i>	
Green Ratio	<i>0.23</i>	<i>0.23</i>		<i>0.23</i>	<i>0.23</i>	<i>0.23</i>	<i>0.65</i>	<i>0.52</i>		<i>0.65</i>	<i>0.52</i>	
Uniform Delay d ₁	<i>25.3</i>	<i>25.6</i>		<i>25.5</i>	<i>25.4</i>	<i>25.9</i>	<i>13.6</i>	<i>17.4</i>		<i>13.6</i>	<i>18.0</i>	
Delay Factor k	<i>0.11</i>	<i>0.11</i>		<i>0.11</i>	<i>0.11</i>	<i>0.11</i>	<i>0.11</i>	<i>0.50</i>		<i>0.14</i>	<i>0.50</i>	
Incremental Delay d ₂	<i>0.6</i>	<i>0.5</i>		<i>0.7</i>	<i>0.4</i>	<i>0.7</i>	<i>0.6</i>	<i>15.2</i>		<i>2.3</i>	<i>19.3</i>	
PF Factor	<i>1.000</i>	<i>1.000</i>		<i>1.000</i>	<i>1.000</i>	<i>1.000</i>	<i>1.000</i>	<i>1.000</i>		<i>1.000</i>	<i>1.000</i>	

Control Delay	25.9	26.1		26.2	25.8	26.7	14.2	32.5		15.8	37.3	
Lane Group LOS	C	C		C	C	C	B	C		B	D	
Approach Delay	26.0			26.2			31.1			34.3		
Approach LOS	C			C			C			C		
Intersection Delay	31.2			Intersection LOS						C		

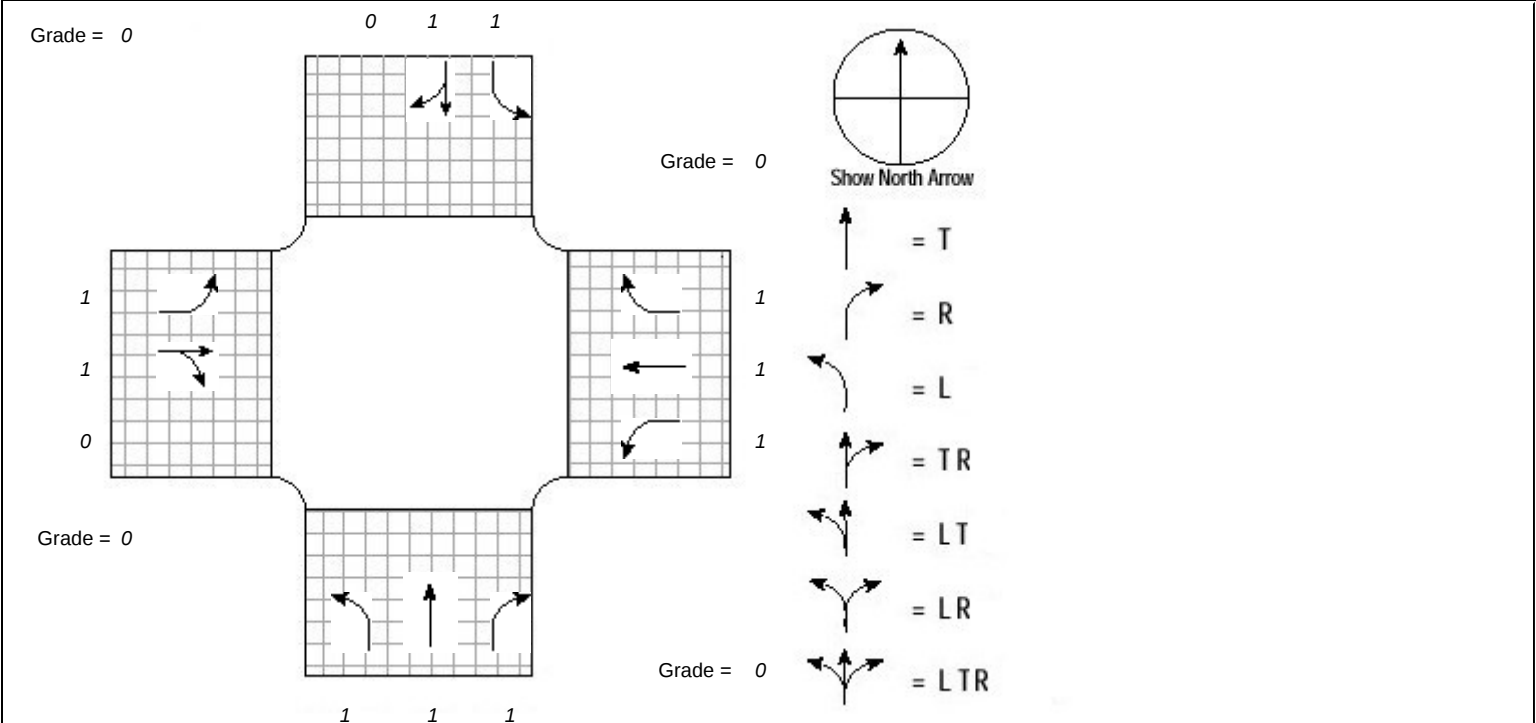
FULL REPORT	
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General Information	
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Analyst	LSB
Agency or Co.	Cathy Sweetapple & Associates
Date Performed	08/16/2008
Time Period	PM Peak Hour

Site Information
Project Name: Client: Location: Date:

Intersection	SW 184 Street/Krome Avenue
Area Type	All other areas
Jurisdiction	Miami-Dade
Analysis Year	Future without Project

[illegible]

Volume and Timing Input

[illegible]

Pedestrian Timing			3.2		3.2		3.2		3.2	
	EW Perm	02	03	04	Excl. Left	NS Perm	07	08		
Timing	G = 25.0	G =	G =	G =	G = 7.0	G = 49.0	G =	G =		
	Y = 5	Y =	Y =	Y =	Y = 3	Y = 5	Y =	Y =		
Duration of Analysis (hrs) = 0.25					Cycle Length C = 94.0					

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	67	33	8	230	27	115	3	711	218	179	829	28
PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adjusted Flow Rate	71	35	8	242	28	121	3	748	229	188	873	29
Lane Group	L	TR		L	T	R	L	T	R	L	TR	
Adjusted Flow Rate	71	43		242	28	121	3	748	229	188	902	
Proportion of LT or RT	0.000	--	0.186	0.000	--	0.000	0.000	--	0.000	0.000	--	0.032
Saturation Flow Rate												
Base Satflow	1900	1900		1900	1900	1900	1900	1900	1900	1900	1900	
Number of Lanes	1	1	0	1	1	1	1	1	1	1	1	0
f_w	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_{HV}	0.980	0.980		0.980	0.980	0.980	0.980	0.926	0.980	0.980	0.928	
f_g	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_p	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_{bb}	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_a	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_{LU}	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_{LT}	0.739	1.000	--	0.729	1.000	--	0.950	1.000	--	0.950	1.000	--
Secondary f_{LT}			--			--	0.077		--	0.147		--

Control Delay	27.0	26.0		35.6	25.8	27.8	17.5	26.7	13.4	20.5	48.9	
Lane Group LOS	C	C		D	C	C	B	C	B	C	D	
Approach Delay	26.6			32.5			23.5			44.0		
Approach LOS	C			C			C			D		
Intersection Delay	33.7			Intersection LOS						C		

FULL REPORT	
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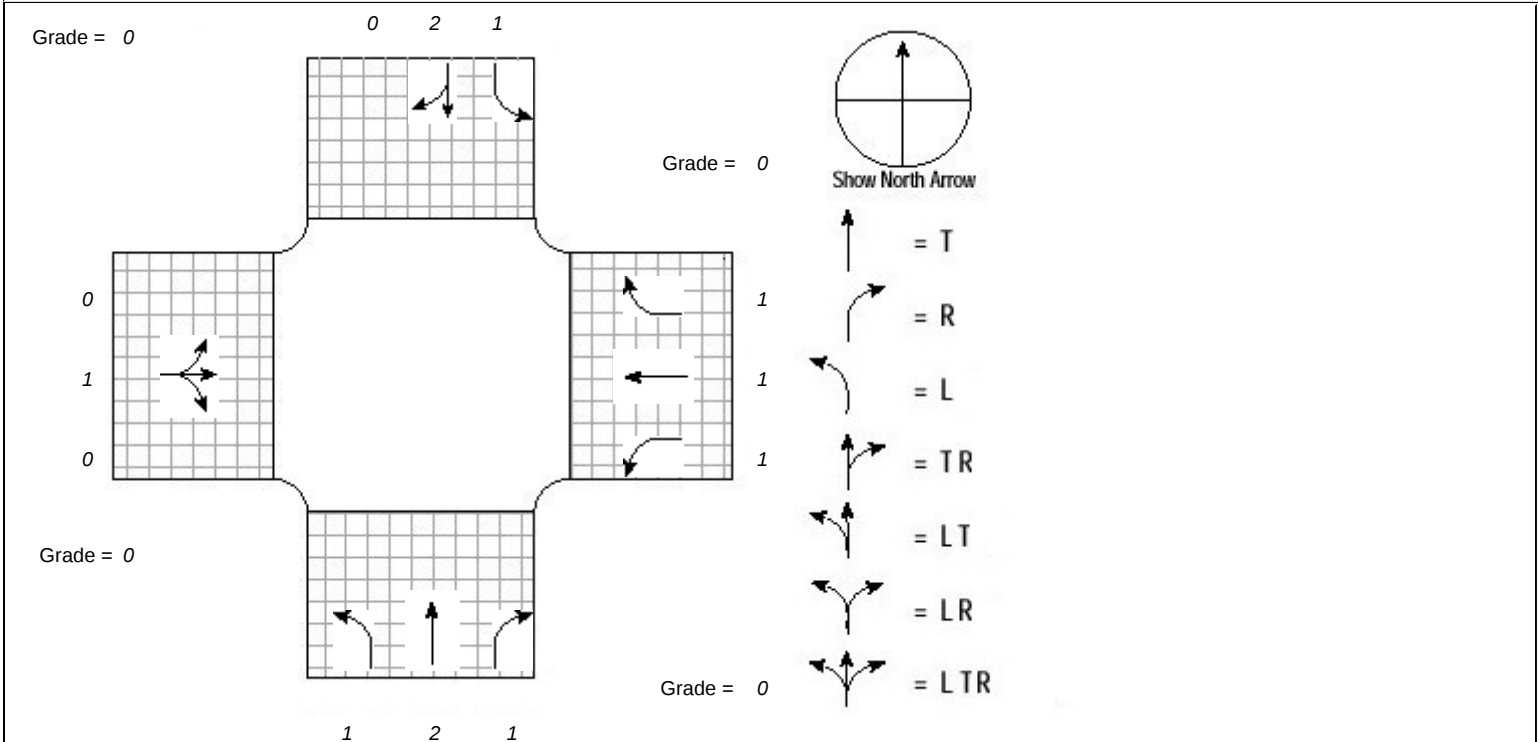
General Information	
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Analyst	LSB
Agency or Co.	Cathy Sweetapple & Associates
Date Performed	08/16/2008
Time Period	PM Peak Hour

Site Information	
Site Name	
Address	
City	
State	
Zip	
County	
Latitude	
Longitude	
Altitude	
Map	
Notes	

Intersection	<i>SW 88 Street/SW 177 Avenue</i>
Area Type	<i>All other areas</i>
Jurisdiction	<i>Miami-Dade</i>
Analysis Year	<i>Future without Project</i>

Intersection Geometry



Volume and Timing Input

[illegible]

Pedestrian Timing			3.2		3.2		3.2		3.2	
	WB Only	EW Perm	03	04	SB Only	NS Perm	07	08		
Timing	G = 15.0	G = 20.0	G =	G =	G = 20.0	G = 38.0	G =	G =		
	Y = 4	Y = 5	Y =	Y =	Y = 3	Y = 5	Y =	Y =		
Duration of Analysis (hrs) = 0.25					Cycle Length C = 110.0					

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	12	13	5	397	3	237	3	443	473	436	553	13
PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adjusted Flow Rate	13	14	5	418	3	249	3	466	393	459	582	14
Lane Group		LTR		L	T	R	L	T	R	L	TR	
Adjusted Flow Rate		32		418	3	249	3	466	393	459	596	
Proportion of LT or RT	0.406	--	0.156	0.000	--	0.000	0.000	--	0.000	0.000	--	0.023
Saturation Flow Rate												
Base Satflow		1900		1900	1900	1900	1900	1900	1900	1900	1900	
Number of Lanes	0	1	0	1	1	1	1	2	1	1	2	0
f_w		1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_{HV}		0.980		0.980	0.980	0.980	0.980	0.926	0.980	0.980	0.927	
f_g		1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_p		1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_{bb}		1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_a		1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_{LU}		1.000		1.000	1.000	1.000	1.000	0.952	1.000	1.000	0.952	
f_{LT}		0.918	--	0.950	1.000	--	0.427	1.000	--	0.950	1.000	--
Secondary f_{LT}			--	0.720	1.000	--			--	0.351		--

f_{RT}	--	0.979		--	1.000	0.850	--	1.000	0.850	--	0.996	
f_{Lpb}		1.000	--	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--
f_{Rpb}	--	1.000		--	1.000	1.000	--	1.000	1.000	--	1.000	
Adjusted Satflow		1674		1770	1863	1583	796	3350	1583	1770	3342	
Secondary Adjusted Satflow			--	1340	1863	--			--	654		--

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane Group		<i>LTR</i>		<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>TR</i>	
Adjusted Flow Rate		32		418	3	249	3	466	393	459	596	
Satflow Rate		1674		1770	1863	1583	796	3350	1583	1770	3342	
Lost Time		2.0		2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Green Ratio		0.18		0.35	0.35	0.35	0.35	0.35	0.35	0.55	0.55	
Lane Group Capacity		304		533	661	561	275	1157	547	566	1853	
v/c Ratio		0.11		0.78	0.00	0.44	0.01	0.40	0.72	0.81	0.32	
Flow Ratio		0.02		0.14	0.00	0.16	0.00	0.14	0.25	0.18	0.18	
Critical Lane Group		<i>N</i>		<i>Y</i>	<i>N</i>	<i>N</i>	<i>N</i>	<i>N</i>	<i>Y</i>	<i>Y</i>	<i>N</i>	
Sum Flow Ratios	0.70											
Lost Time/Cycle	15.00											
Critical v/c Ratio	0.81											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane Group		<i>LTR</i>		<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>TR</i>	
Adjusted Flow Rate		32		418	3	249	3	466	393	459	596	
Lane Group Capacity		304		533	661	561	275	1157	547	566	1853	
v/c Ratio		0.11		0.78	0.00	0.44	0.01	0.40	0.72	0.81	0.32	
Green Ratio		0.18		0.35	0.35	0.35	0.35	0.35	0.35	0.55	0.55	
Uniform Delay d ₁		37.5		32.0	23.0	27.2	23.7	27.4	31.3	15.9	13.3	
Delay Factor k		0.11		0.33	0.11	0.11	0.11	0.11	0.28	0.35	0.11	
Incremental Delay d ₂		0.2		7.6	0.0	0.6	0.0	0.2	4.5	8.7	0.1	

PF Factor		1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
Control Delay		37.7		39.5	23.0	27.8	23.7	27.6	35.9	24.6	13.4	
Lane Group LOS		D		D	C	C	C	C	D	C	B	
Approach Delay	37.7			35.1			31.4			18.3		
Approach LOS	D			D			C			B		
Intersection Delay	27.1			Intersection LOS						C		

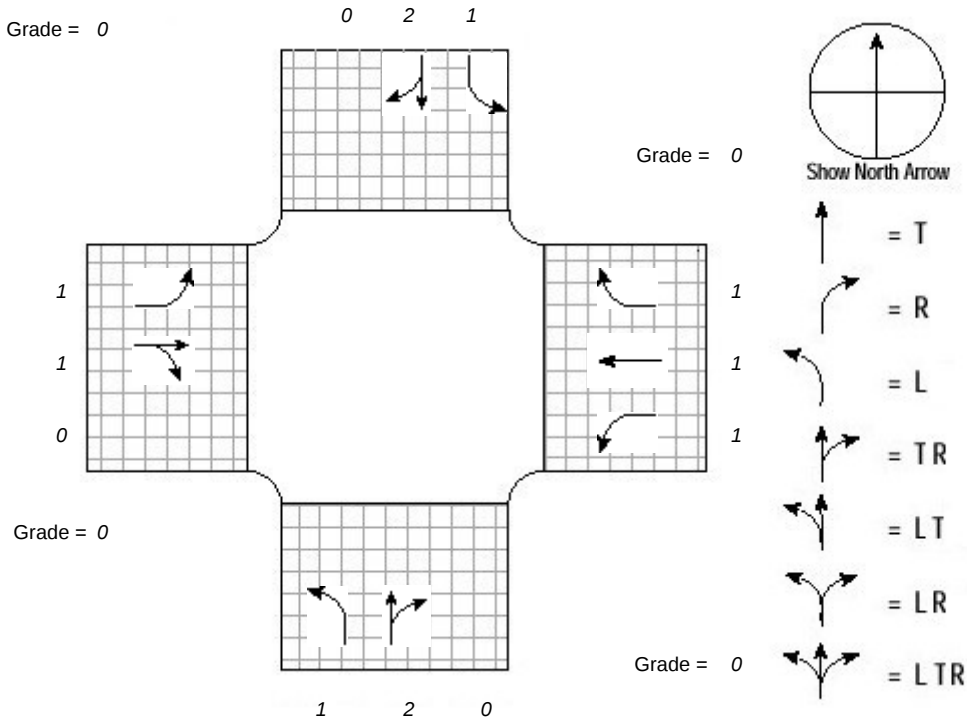
TWO-WAY STOP CONTROL SUMMARY

General Information								Site Information							
Analyst <i>LSB</i>				Intersection <i>SW 136 Street/SW 157 Avenue</i>				Agency/Co. <i>Cathy Sweetapple & Associates</i>				Jurisdiction <i>Miami-Dade</i>			
Date Performed <i>08/20/2008</i>				Analysis Year <i>Future Without Project</i>				Analysis Time Period <i>PM Peak Hour</i>							
Project Description <i>Parkland DRI</i>															
East/West Street: <i>SW 136 Street</i>						North/South Street: <i>SW 157 Avenue</i>									
Intersection Orientation: <i>North-South</i>						Study Period (hrs): <i>0.25</i>									
Vehicle Volumes and Adjustments															
Major Street		Northbound				Southbound									
Movement		1		2		3		4		5		6			
		L		T		R		L		T		R			
Volume (veh/h)		55				219									
Peak-Hour Factor, PHF		0.95		0.95		0.95		0.95		0.95		0.95			
Hourly Flow Rate, HFR (veh/h)		0		52		60		529		46		0			
Percent Heavy Vehicles		2		--		--		2		--		--			
Median Type		<i>Undivided</i>													
RT Channelized						0						0			
Lanes		1		0		1		0		0		0			
Configuration		L				R									
Upstream Signal				0						0					
Minor Street		Eastbound				Westbound									
Movement		7		8		9		10		11		12			
		L		T		R		L		T		R			
Volume (veh/h)				50		57		503		44					
Peak-Hour Factor, PHF		0.95		0.95		0.95		0.95		0.95		0.95			
Hourly Flow Rate, HFR (veh/h)		0		0		0		57		0		230			
Percent Heavy Vehicles		2		2		2		2		2		2			
Percent Grade (%)				0						0					
Flared Approach				N						N					
Storage				0						0					
RT Channelized						0						0			
Lanes		0		2		1		1		2		0			
Configuration				T		R		L		T					
Delay, Queue Length, and Level of Service															
Approach	Northbound	Southbound	Westbound			Eastbound									
Movement	1	4	7	8	9	10	11	12							
Lane Configuration	L		L	T			T	R							
v (veh/h)	57		529	46			52	60							
C (m) (veh/h)	1623		678	749			559	1085							
v/c	0.04		0.78	0.06			0.09	0.06							
95% queue length	0.11		7.57	0.20			0.31	0.18							
Control Delay (s/veh)	7.3		26.5	10.1			12.1	8.5							
LOS	A		D	B			B	A							
Approach Delay (s/veh)	--	--	25.2			10.2									
Approach LOS	--	--	D			B									

FULL REPORT

General Information				Site Information	
Analyst	LSB			Intersection	SW 152 Street/SW 157 Avenue
Agency or Co.	Cathy Sweetapple & Associates			Area Type	All other areas
Date Performed	08/20/2008			Jurisdiction	Miami-Dade
Time Period	PM Peak Hour			Analysis Year	Future without Project

Intersection Geometry



Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume (vph)	15	90	25	76	137	183	42	99	93	242	207	41
% Heavy Veh	2	2	2	2	2	2	2	2	2	2	2	2
PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Actuated (P/A)	A	P	P	A	P	P	A	A	A	A	A	A
Startup Lost Time	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Extension of Effective Green	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Arrival Type	3	3		3	3	3	3	3		3	3	
Unit Extension	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Ped/Bike/RTOR Volume	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	12.0	12.0		12.0	12.0	12.0	12.0	12.0		12.0	12.0	
Parking (Y or N)	N		N	N		N	N		N	N		N
Parking/Hour												
Bus Stops/Hour	0	0		0	0	0	0	0		0	0	

Pedestrian Timing			3.2		3.2		3.2		3.2	
	Excl. Left	EW Perm	03	04	Excl. Left	NS Perm	07	08		
Timing	G = 10.0	G = 40.0	G =	G =	G = 20.0	G = 22.0	G =	G =		
	Y = 4	Y = 5	Y =	Y =	Y = 4	Y = 5	Y =	Y =		
Duration of Analysis (hrs) = 0.25							Cycle Length C = 110.0			

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	15	90	25	76	137	183	42	99	93	242	207	41
PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adjusted Flow Rate	16	95	26	80	144	193	44	104	98	255	218	43
Lane Group	L	TR		L	T	R	L	TR		L	TR	
Adjusted Flow Rate	16	121		80	144	193	44	202		255	261	
Proportion of LT or RT	0.000	--	0.215	0.000	--	0.000	0.000	--	0.485	0.000	--	0.165
Saturation Flow Rate												
Base Satflow	1900	1900		1900	1900	1900	1900	1900		1900	1900	
Number of Lanes	1	1	0	1	1	1	1	2	0	1	2	0
f_w	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
f_{HV}	0.980	0.980		0.980	0.980	0.980	0.980	0.980		0.980	0.980	
f_g	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
f_p	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
f_{bb}	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
f_a	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
f_{LU}	1.000	1.000		1.000	1.000	1.000	1.000	0.952		1.000	0.952	
f_{LT}	0.950	1.000	--	0.950	1.000	--	0.950	1.000	--	0.950	1.000	--
Secondary f_{LT}	0.573	0.573	--	0.601		--	0.426		--	0.493		--

f_{RT}	--	0.968		--	1.000	0.850	--	0.927		--	0.975	
f_{Lpb}	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--
f_{Rpb}	--	1.000		--	1.000	1.000	--	1.000		--	1.000	
Adjusted Satflow	1770	1803		1770	1863	1583	1770	3289		1770	3459	
Secondary Adjusted Satflow	1068	1033	--	1120		--	794		--	918		--

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>TR</i>		<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>	
Adjusted Flow Rate	16	121		80	144	193	44	202		255	261	
Satflow Rate	1770	1803		1770	1863	1583	1770	3289		1770	3459	
Lost Time	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Green Ratio	0.49	0.36		0.49	0.36	0.36	0.42	0.20		0.42	0.20	
Lane Group Capacity	588	656		609	677	576	510	658		539	692	
v/c Ratio	0.03	0.18		0.13	0.21	0.34	0.09	0.31		0.47	0.38	
Flow Ratio	0.01	0.07		0.05	0.08	0.12	0.02	0.06		0.14	0.08	
Critical Lane Group	<i>N</i>	<i>N</i>		<i>Y</i>	<i>N</i>	<i>Y</i>	<i>N</i>	<i>N</i>		<i>Y</i>	<i>Y</i>	
Sum Flow Ratios	0.39											
Lost Time/Cycle	20.00											
Critical v/c Ratio	0.47											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>TR</i>		<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>	
Adjusted Flow Rate	16	121		80	144	193	44	202		255	261	
Lane Group Capacity	588	656		609	677	576	510	658		539	692	
v/c Ratio	0.03	0.18		0.13	0.21	0.34	0.09	0.31		0.47	0.38	
Green Ratio	0.49	0.36		0.49	0.36	0.36	0.42	0.20		0.42	0.20	
Uniform Delay d ₁	14.6	23.9		15.1	24.1	25.4	19.3	37.5		21.9	38.1	
Delay Factor k	0.11	0.50		0.11	0.50	0.50	0.11	0.11		0.11	0.11	
Incremental Delay d ₂	0.0	0.6		0.1	0.7	1.6	0.1	0.3		0.7	0.3	
PF Factor	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	

Control Delay	14.6	24.5		15.2	24.9	26.9	19.4	37.8		22.6	38.4	
Lane Group LOS	B	C		B	C	C	B	D		C	D	
Approach Delay	23.3			24.0			34.5			30.6		
Approach LOS	C			C			C			C		
Intersection Delay	28.5			Intersection LOS						C		

FULL REPORT	
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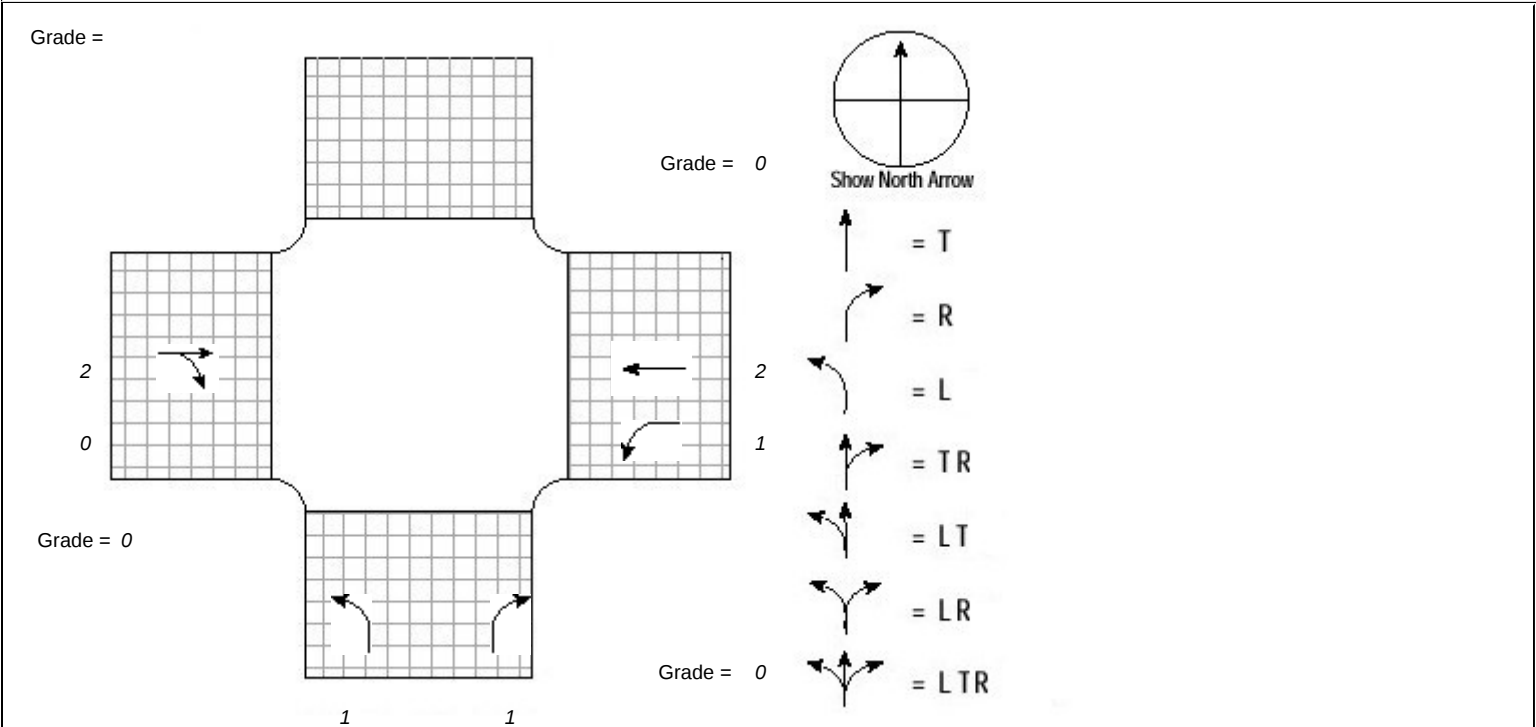
General Information	
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Analyst	LSB
Agency or Co.	Cathy Sweetapple & Associates
Date Performed	08/20/2008
Time Period	PM Peak Hour

Site Information	
Site Name	
Address	
City	
State	
Zip	
County	
Latitude	
Longitude	
Altitude	
Map	
Notes	

Intersection	SW 152 Street/SW 147 Avenue
Area Type	All other areas
Jurisdiction	Miami-Dade
Analysis Year	Future without Project

Intersection Geometry	
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Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume (vph)		604	27	476	843		27		345			
% Heavy Veh		2	2	2	2		2		2			
PHF		0.95	0.95	0.95	0.95		0.95		0.95			
Actuated (P/A)		P	P	A	P		A		A			
Startup Lost Time		2.0		2.0	2.0		2.0		2.0			
Extension of Effective Green		2.0		2.0	2.0		2.0		2.0			
Arrival Type		3		3	3		3		3			
Unit Extension		3.0		3.0	3.0		3.0		3.0			
Ped/Bike/RTOR Volume	0	0	0	0	0		0	0	100			
Lane Width		12.0		12.0	12.0		12.0		12.0			
Parking (Y or N)	N		N	N		N	N		N			
Parking/Hour												
Bus Stops/Hour		0		0	0		0		0			

Pedestrian Timing			3.2		3.2		3.2			
	WB Only	EW Perm	03	04	NB Only	06	07	08		
Timing	G = 12.0	G = 40.0	G =	G =	G = 20.0	G =	G =	G =		
	Y = 3	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =		
Duration of Analysis (hrs) = 0.25					Cycle Length C = 85.0					

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume		604	27	476	843		27		345			
PHF		0.95	0.95	0.95	0.95		0.95		0.95			
Adjusted Flow Rate		636	28	501	887		28		258			
Lane Group		TR		L	T		L		R			
Adjusted Flow Rate		664		501	887		28		258			
Proportion of LT or RT	0.000	--	0.042	0.000	--	0.000		--			--	
Saturation Flow Rate												
Base Satflow		1900		1900	1900		1900		1900			
Number of Lanes		2	0	1	2		1		1			
f_w		1.000		1.000	1.000		1.000		1.000			
f_{HV}		0.980		0.980	0.980		0.980		0.980			
f_g		1.000		1.000	1.000		1.000		1.000			
f_p		1.000		1.000	1.000		1.000		1.000			
f_{bb}		1.000		1.000	1.000		1.000		1.000			
f_a		1.000		1.000	1.000		1.000		1.000			
f_{LU}		0.952		1.000	0.952		1.000		1.000			
f_{LT}		1.000	--	0.950	1.000	--	0.950		--			--
Secondary f_{LT}			--	0.299	0.829	--			--			--

f _{RT}	--	0.994		--	1.000		--		0.850	--		
f _{Lpb}		1.000	--	1.000	1.000	--	1.000		--			--
f _{Rpb}	--	1.000		--	1.000		--		1.000	--		
Adjusted Satflow		3524		1770	3547		1770		1583			
Secondary Adjusted Satflow			--	558	2941	--			--			--

General Information

Capacity Analysis

Lane Group Capacity, Control Delay, and LOS Determination

PF Factor		1.000		1.000	1.000		1.000		1.000			
Control Delay		15.4		34.8	7.6		25.3		35.2			
Lane Group LOS		B		C	A		C		D			
Approach Delay	15.4			17.4			34.2					
Approach LOS	B			B			C					
Intersection Delay	18.9			Intersection LOS						B		

Pedestrian Timing			3.2		3.2		3.2		3.2	
	Excl. Left	Thru & RT	03	04	Excl. Left	Thru & RT	07	08		
Timing	G = 21.0	G = 50.0	G =	G =	G = 19.0	G = 50.0	G =	G =		
	Y = 4	Y = 6	Y =	Y =	Y = 4	Y = 6	Y =	Y =		
Duration of Analysis (hrs) = 0.25					Cycle Length C = 160.0					

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	314	737	124	669	1241	785	319	932	445	597	1245	509
PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adjusted Flow Rate	331	776	131	704	1306	826	336	981	468	628	1311	536
Lane Group	L	TR		L	T	R	L	T	R	L	T	R
Adjusted Flow Rate	331	907		704	1306	826	336	981	468	628	1311	536
Proportion of LT or RT	0.000	--	0.144	0.000	--	0.000	0.000	--	0.000	0.000	--	0.000
Saturation Flow Rate												
Base Satflow	1900	1900		1900	1900	1900	1900	1900	1900	1900	1900	1900
Number of Lanes	2	3	0	2	2	1	2	2	1	2	2	1
f_w	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_{HV}	0.980	0.980		0.980	0.980	0.980	0.980	0.980	0.980	0.980	0.980	0.980
f_g	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_p	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_{bb}	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_a	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_{LU}	0.971	0.908		0.971	0.952	1.000	0.971	0.952	1.000	0.971	0.952	1.000
f_{LT}	0.950	1.000	--	0.950	1.000	--	0.950	1.000	--	0.950	1.000	--
Secondary f_{LT}			--			--			--			--

f_{RT}	--	0.978		--	1.000	0.850	--	1.000	0.850	--	1.000	0.850
f_{Lpb}	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--
f_{Rpb}	--	1.000		--	1.000	1.000	--	1.000	1.000	--	1.000	1.000
Adjusted Satflow	3437	4964		3437	3547	1583	3437	3547	1583	3437	3547	1583
Secondary Adjusted Satflow			--			--			--			--

CAPACITY AND LOS WORKSHEET

General Information

Project Description *Parkland DRI*

Capacity Analysis

	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>TR</i>		<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>
Adjusted Flow Rate	331	907		704	1306	826	336	981	468	628	1311	536
Satflow Rate	3437	4964		3437	3547	1583	3437	3547	1583	3437	3547	1583
Lost Time	2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Green Ratio	0.13	0.31		0.13	0.31	0.31	0.12	0.31	0.31	0.12	0.31	0.31
Lane Group Capacity	451	1551		451	1108	495	408	1108	495	408	1108	495
v/c Ratio	0.73	0.58		1.56	1.18	1.67	0.82	0.89	0.95	1.54	1.18	1.08
Flow Ratio	0.10	0.18		0.20	0.37	0.52	0.10	0.28	0.30	0.18	0.37	0.34
Critical Lane Group	<i>N</i>	<i>N</i>		<i>Y</i>	<i>N</i>	<i>Y</i>	<i>N</i>	<i>N</i>	<i>N</i>	<i>Y</i>	<i>Y</i>	<i>N</i>
Sum Flow Ratios	1.28											
Lost Time/Cycle	20.00											
Critical v/c Ratio	1.46											

Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>TR</i>		<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>
Adjusted Flow Rate	331	907		704	1306	826	336	981	468	628	1311	536
Lane Group Capacity	451	1551		451	1108	495	408	1108	495	408	1108	495
v/c Ratio	0.73	0.58		1.56	1.18	1.67	0.82	0.89	0.95	1.54	1.18	1.08
Green Ratio	0.13	0.31		0.13	0.31	0.31	0.12	0.31	0.31	0.12	0.31	0.31
Uniform Delay d_1	66.8	46.3		69.5	55.0	55.0	68.9	52.3	53.7	70.5	55.0	55.0
Delay Factor k	0.29	0.18		0.50	0.50	0.50	0.36	0.50	0.50	0.50	0.50	0.50
	6.1	0.6		263.1	90.0	309.7	12.8	10.4	28.9	254.6	91.9	64.7

Incremental Delay d ₂												
PF Factor	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Control Delay	72.9	46.8		332.6	145.0	364.7	81.7	62.7	82.6	325.1	146.9	119.7
Lane Group LOS	E	D		F	F	F	F	E	F	F	F	F
Approach Delay	53.8			255.6			71.5			186.2		
Approach LOS	D			F			E			F		
Intersection Delay	165.6			Intersection LOS						F		

FULL REPORT	
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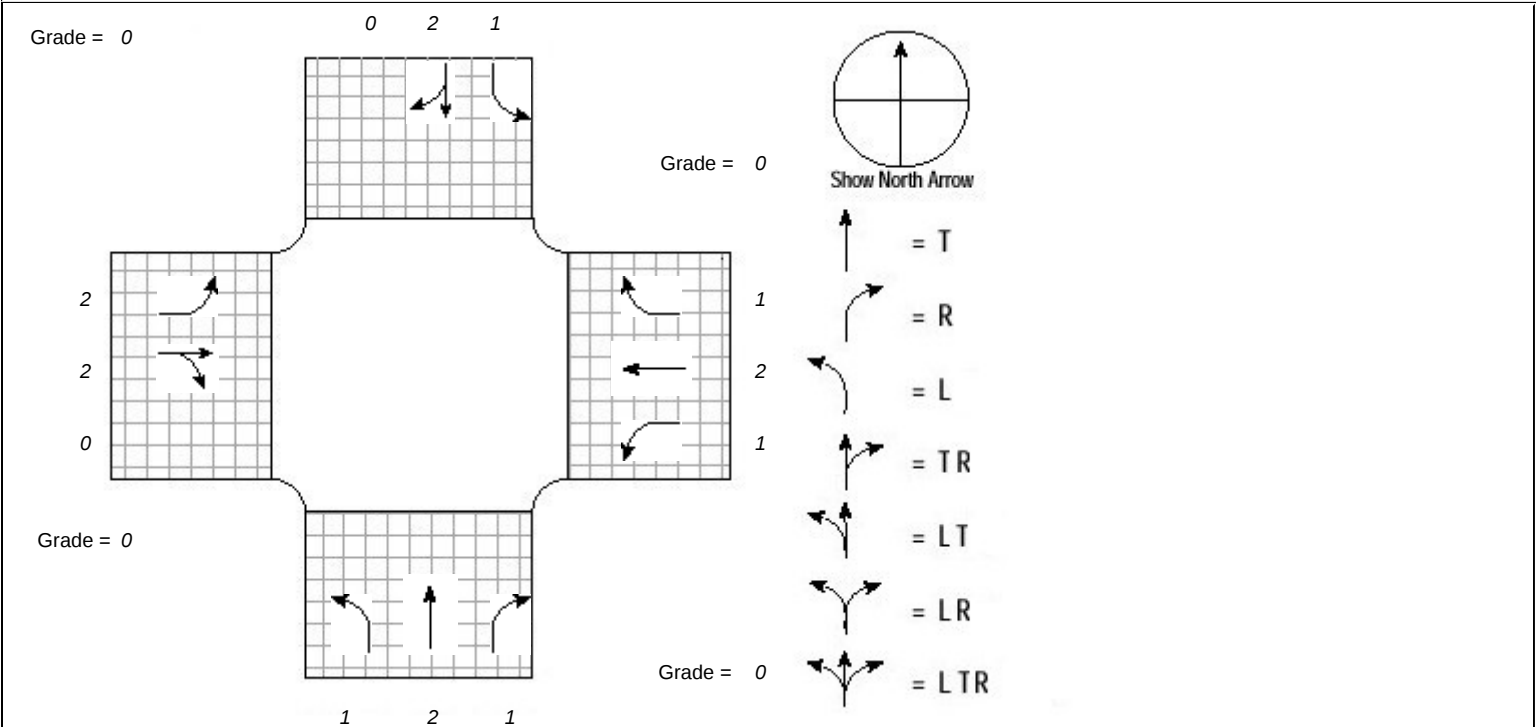
General Information	
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Analyst	LSB
Agency or Co.	Cathy Sweetapple & Associates
Date Performed	08/20/2008
Time Period	PM Peak Hour

Site Information

Intersection	SW 152 Street/SW 117 Avenue
Area Type	All other areas
Jurisdiction	Miami-Dade
Analysis Year	Future without Project

Intersection Geometry



Volume and Timing Input

[illegible]

Pedestrian Timing			3.2		3.2		3.2		3.2	
	Excl. Left	EB Only	Thru & RT	04	Excl. Left	NB Only	Thru & RT	08		
Timing	G = 10.0	G = 15.0	G = 56.0	G =	G = 20.0	G = 12.0	G = 25.0	G =		
	Y = 3	Y = 3	Y = 5	Y =	Y = 3	Y = 3	Y = 5	Y =		
Duration of Analysis (hrs) = 0.25					Cycle Length C = 160.0					

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	387	1638	271	40	1526	19	285	337	82	87	483	100
PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adjusted Flow Rate	407	1724	285	42	1606	20	300	355	86	92	508	105
Lane Group	L	TR		L	T	R	L	T	R	L	TR	
Adjusted Flow Rate	407	2009		42	1606	20	300	355	86	92	613	
Proportion of LT or RT	0.000	--	0.142	0.000	--	0.000	0.000	--	0.000	0.000	--	0.171
Saturation Flow Rate												
Base Satflow	1900	1900		1900	1900	1900	1900	1900	1900	1900	1900	
Number of Lanes	2	2	0	1	2	1	1	2	1	1	2	0
f_w	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_{HV}	0.980	0.980		0.980	0.980	0.980	0.980	0.980	0.980	0.980	0.980	
f_g	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_p	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_{bb}	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_a	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_{LU}	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_{LT}	0.950	1.000	--	0.950	1.000	--	0.950	1.000	--	0.950	1.000	--
Secondary f_{LT}			--			--			--			--

f_{RT}	--	0.979		--	1.000	0.850	--	1.000	0.850	--	0.974	
f_{Lpb}	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--
f_{Rpb}	--	1.000		--	1.000	1.000	--	1.000	1.000	--	1.000	
Adjusted Satflow	3539	3646		1770	3725	1583	1770	3725	1583	1770	3630	
Secondary Adjusted Satflow			--			--			--			--

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>TR</i>		<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>TR</i>	
Adjusted Flow Rate	<i>407</i>	<i>2009</i>		<i>42</i>	<i>1606</i>	<i>20</i>	<i>300</i>	<i>355</i>	<i>86</i>	<i>92</i>	<i>613</i>	
Satflow Rate	<i>3539</i>	<i>3646</i>		<i>1770</i>	<i>3725</i>	<i>1583</i>	<i>1770</i>	<i>3725</i>	<i>1583</i>	<i>1770</i>	<i>3630</i>	
Lost Time	<i>2.0</i>	<i>2.0</i>		<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	
Green Ratio	<i>0.17</i>	<i>0.46</i>		<i>0.06</i>	<i>0.35</i>	<i>0.35</i>	<i>0.22</i>	<i>0.25</i>	<i>0.25</i>	<i>0.13</i>	<i>0.16</i>	
Lane Group Capacity	<i>619</i>	<i>1686</i>		<i>111</i>	<i>1304</i>	<i>554</i>	<i>387</i>	<i>931</i>	<i>396</i>	<i>221</i>	<i>567</i>	
v/c Ratio	<i>0.66</i>	<i>1.19</i>		<i>0.38</i>	<i>1.23</i>	<i>0.04</i>	<i>0.78</i>	<i>0.38</i>	<i>0.22</i>	<i>0.42</i>	<i>1.08</i>	
Flow Ratio	<i>0.12</i>	<i>0.55</i>		<i>0.02</i>	<i>0.43</i>	<i>0.01</i>	<i>0.17</i>	<i>0.10</i>	<i>0.05</i>	<i>0.05</i>	<i>0.17</i>	
Critical Lane Group	<i>N</i>	<i>Y</i>		<i>Y</i>	<i>N</i>	<i>N</i>	<i>Y</i>	<i>N</i>	<i>N</i>	<i>N</i>	<i>Y</i>	
Sum Flow Ratios	<i>0.91</i>											
Lost Time/Cycle	<i>16.00</i>											
Critical v/c Ratio	<i>1.01</i>											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>TR</i>		<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>TR</i>	
Adjusted Flow Rate	<i>407</i>	<i>2009</i>		<i>42</i>	<i>1606</i>	<i>20</i>	<i>300</i>	<i>355</i>	<i>86</i>	<i>92</i>	<i>613</i>	
Lane Group Capacity	<i>619</i>	<i>1686</i>		<i>111</i>	<i>1304</i>	<i>554</i>	<i>387</i>	<i>931</i>	<i>396</i>	<i>221</i>	<i>567</i>	
v/c Ratio	<i>0.66</i>	<i>1.19</i>		<i>0.38</i>	<i>1.23</i>	<i>0.04</i>	<i>0.78</i>	<i>0.38</i>	<i>0.22</i>	<i>0.42</i>	<i>1.08</i>	
Green Ratio	<i>0.17</i>	<i>0.46</i>		<i>0.06</i>	<i>0.35</i>	<i>0.35</i>	<i>0.22</i>	<i>0.25</i>	<i>0.25</i>	<i>0.13</i>	<i>0.16</i>	
Uniform Delay d ₁	<i>61.5</i>	<i>43.0</i>		<i>72.0</i>	<i>52.0</i>	<i>34.2</i>	<i>58.8</i>	<i>49.7</i>	<i>47.6</i>	<i>64.6</i>	<i>67.5</i>	
Delay Factor k	<i>0.23</i>	<i>0.50</i>		<i>0.11</i>	<i>0.50</i>	<i>0.50</i>	<i>0.32</i>	<i>0.11</i>	<i>0.11</i>	<i>0.11</i>	<i>0.50</i>	
	<i>2.6</i>	<i>92.4</i>		<i>2.2</i>	<i>111.1</i>	<i>0.1</i>	<i>9.5</i>	<i>0.3</i>	<i>0.3</i>	<i>1.3</i>	<i>61.6</i>	

Incremental Delay d_2												
PF Factor	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
Control Delay	64.1	135.4		74.2	163.1	34.4	68.3	50.0	47.9	65.9	129.1	
Lane Group LOS	E	F		E	F	C	E	D	D	E	F	
Approach Delay	123.4			159.3			57.2			120.8		
Approach LOS	F			F			E			F		
Intersection Delay	125.0			Intersection LOS						F		

FULL REPORT	
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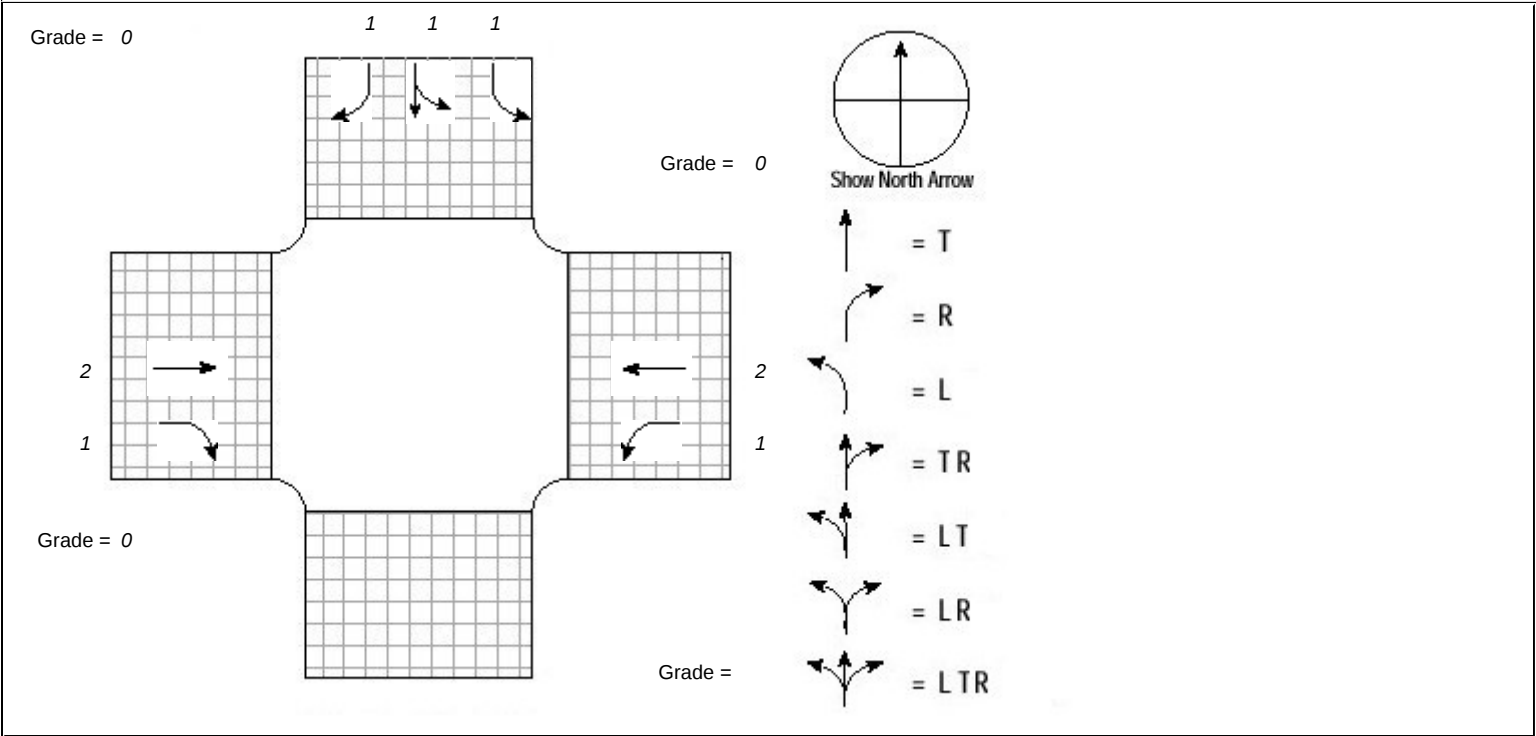
General Information	
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Analyst	LSB
Agency or Co.	Cathy Sweetapple & Associates
Date Performed	08/20/2008
Time Period	PM Peak Hour

Site Information

Intersection	SW 152 Street/SR 821 W
Area Type	All other areas
Jurisdiction	Miami-Dade
Analysis Year	Future without Project

Intersection Geometry

[illegible]

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume (vph)		1453	388	209	1342					421	107	248
% Heavy Veh		2	2	2	2					2	2	2
PHF		0.95	0.95	0.95	0.95					0.95	0.95	0.95
Actuated (P/A)		A	A	A	A					P	P	P
Startup Lost Time		2.0	2.0	2.0	2.0					2.0	2.0	2.0
Extension of Effective Green		2.0	2.0	2.0	2.0					2.0	2.0	2.0
Arrival Type		3	3	3	3					3	3	3
Unit Extension		3.0	3.0	3.0	3.0					3.0	3.0	3.0
Ped/Bike/RTOR Volume	0	0	0	0	0					0	0	0
Lane Width		12.0	12.0	12.0	12.0					12.0	12.0	12.0
Parking (Y or N)	N		N	N		N				N		N
Parking/Hour												
Bus Stops/Hour		0	0	0	0					0	0	0

Pedestrian Timing			3.2		3.2		3.2	
	WB Only	EW Perm	03	04	SB Only	06	07	08
Timing	G = 11.0	G = 50.0	G =	G =	G = 35.0	G =	G =	G =
	Y = 3	Y = 5	Y =	Y =	Y = 6	Y =	Y =	Y =
Duration of Analysis (hrs) = 0.25					Cycle Length C = 110.0			

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume		1453	388	209	1342					421	107	248
PHF		0.95	0.95	0.95	0.95					0.95	0.95	0.95
Adjusted Flow Rate		1529	408	220	1413					443	113	261
Lane Group		T	R	L	T					L	LT	R
Adjusted Flow Rate		1529	408	220	1413					443	113	261
Proportion of LT or RT	0.000	--	0.000	0.000	--	0.000		--		0.000	--	0.000
Saturation Flow Rate												
Base Satflow		1900	1900	1900	1900					1900	1900	1900
Number of Lanes		2	1	1	2					1	1	1
f_w		1.000	1.000	1.000	1.000					1.000	1.000	1.000
f_{HV}		0.980	0.980	0.980	0.980					0.980	0.980	0.980
f_g		1.000	1.000	1.000	1.000					1.000	1.000	1.000
f_p		1.000	1.000	1.000	1.000					1.000	1.000	1.000
f_{bb}		1.000	1.000	1.000	1.000					1.000	1.000	1.000
f_a		1.000	1.000	1.000	1.000					1.000	1.000	1.000
f_{LU}		0.952	1.000	1.000	0.952					1.000	1.000	1.000
f_{LT}		1.000	--	0.950	1.000	--			--	0.950	1.000	--
Secondary f_{LT}			--	0.075	0.499	--			--			--

f_{RT}	--	1.000	0.850	--	1.000		--			--	1.000	0.850
f_{Lpb}		1.000	--	1.000	1.000	--			--	1.000	1.000	--
f_{Rpb}	--	1.000	1.000	--	1.000		--			--	1.000	1.000
Adjusted Satflow		3547	1583	1770	3547					1770	1863	1583
Secondary Adjusted Satflow			--	141	1770	--			--			--

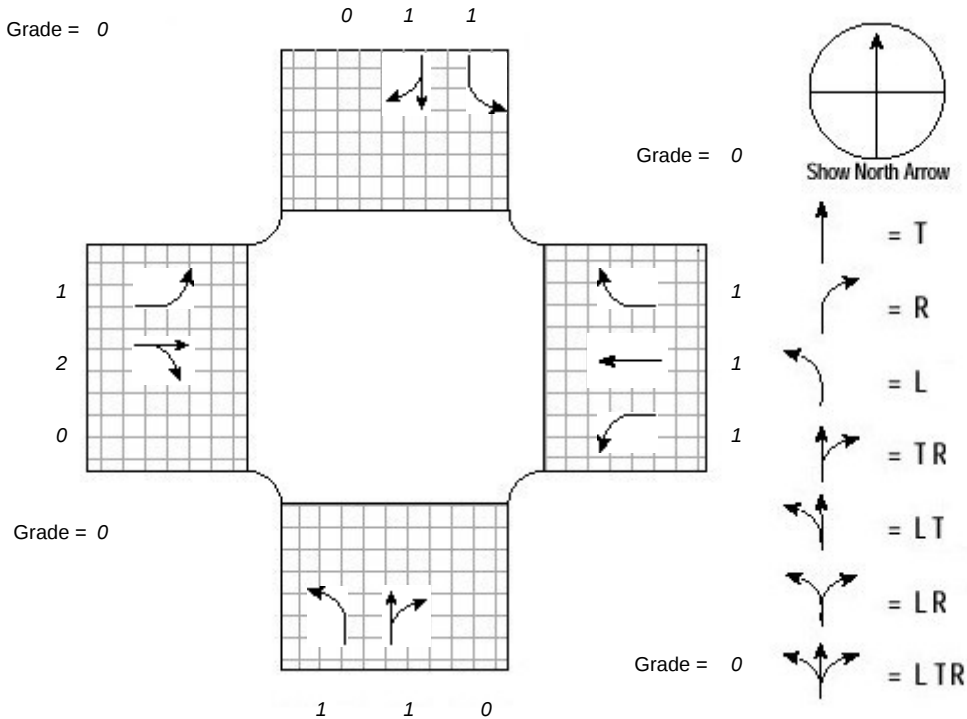
CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane Group		<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>					<i>L</i>	<i>LT</i>	<i>R</i>
Adjusted Flow Rate		1529	408	220	1413					443	113	261
Satflow Rate		3547	1583	1770	3547					1770	1863	1583
Lost Time		2.0	2.0	2.0	2.0					2.0	2.0	2.0
Green Ratio		0.45	0.45	0.58	0.58					0.32	0.32	0.32
Lane Group Capacity		1612	720	245	2064					563	593	504
v/c Ratio		0.95	0.57	0.90	0.68					0.79	0.19	0.52
Flow Ratio		0.43	0.26	0.10	0.40					0.25	0.06	0.16
Critical Lane Group		Y	N	Y	N					Y	N	N
Sum Flow Ratios	0.78											
Lost Time/Cycle	16.00											
Critical v/c Ratio	0.91											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane Group		<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>					<i>L</i>	<i>LT</i>	<i>R</i>
Adjusted Flow Rate		1529	408	220	1413					443	113	261
Lane Group Capacity		1612	720	245	2064					563	593	504
v/c Ratio		0.95	0.57	0.90	0.68					0.79	0.19	0.52
Green Ratio		0.45	0.45	0.58	0.58					0.32	0.32	0.32
Uniform Delay d ₁		28.8	22.0	32.4	16.0					34.1	27.2	30.6
Delay Factor k		0.46	0.16	0.42	0.25					0.50	0.50	0.50
		12.3	1.1	32.0	1.0					10.6	0.7	3.8

Incremental Delay d_2												
PF Factor		1.000	1.000	1.000	1.000					1.000	1.000	1.000
Control Delay		41.1	23.1	64.4	16.9					44.7	27.9	34.4
Lane Group LOS		D	C	E	B					D	C	C
Approach Delay	37.3			23.3						39.1		
Approach LOS	D			C						D		
Intersection Delay	32.4			Intersection LOS						C		

FULL REPORT

General Information				Site Information	
Analyst	LSB			Intersection	SW 184 Street/SW 147 Avenue
Agency or Co.	Cathy Sweetapple & Associates			Area Type	All other areas
Date Performed	08/20/2008			Jurisdiction	Miami-Dade
Time Period	PM Peak Hour			Analysis Year	Future without Project

Intersection Geometry



Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume (vph)	46	326	87	20	138	88	86	264	71	84	274	29
% Heavy Veh	2	2	2	2	2	2	2	2	2	2	2	2
PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Actuated (P/A)	P	P	P	P	P	P	A	A	A	A	A	A
Startup Lost Time	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Extension of Effective Green	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	
Arrival Type	3	3		3	3	3	3	3		3	3	
Unit Extension	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Ped/Bike/RTOR Volume	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	12.0	12.0		12.0	12.0	12.0	12.0	12.0		12.0	12.0	
Parking (Y or N)	N		N	N		N	N		N	N		N
Parking/Hour												
Bus Stops/Hour	0	0		0	0	0	0	0		0	0	

Pedestrian Timing			3.2		3.2		3.2		3.2	
	EW Perm	02	03	04	NS Perm	06	07	08		
Timing	G = 51.0	G =	G =	G =	G = 50.0	G =	G =	G =		
	Y = 5	Y =	Y =	Y =	Y = 4	Y =	Y =	Y =		
Duration of Analysis (hrs) = 0.25					Cycle Length C = 110.0					

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	46	326	87	20	138	88	86	264	71	84	274	29
PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adjusted Flow Rate	48	343	92	21	145	93	91	278	75	88	288	31
Lane Group	L	TR		L	T	R	L	TR		L	TR	
Adjusted Flow Rate	48	435		21	145	93	91	353		88	319	
Proportion of LT or RT	0.000	--	0.211	0.000	--	0.000	0.000	--	0.212	0.000	--	0.097
Saturation Flow Rate												
Base Satflow	1900	1900		1900	1900	1900	1900	1900		1900	1900	
Number of Lanes	1	2	0	1	1	1	1	1	0	1	1	0
f_w	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
f_{HV}	0.980	0.980		0.980	0.980	0.980	0.980	0.980		0.980	0.980	
f_g	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
f_p	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
f_{bb}	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
f_a	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
f_{LU}	1.000	0.952		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
f_{LT}	0.662	1.000	--	0.465	1.000	--	0.466	1.000	--	0.433	1.000	--
Secondary f_{LT}			--			--			--			--

f_{RT}	--	0.968		--	1.000	0.850	--	0.968		--	0.985	
f_{Lpb}	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--
f_{Rpb}	--	1.000		--	1.000	1.000	--	1.000		--	1.000	
Adjusted Satflow	1234	3434		865	1863	1583	868	1803		806	1836	
Secondary Adjusted Satflow			--			--			--			--

General Information

Capacity Analysis

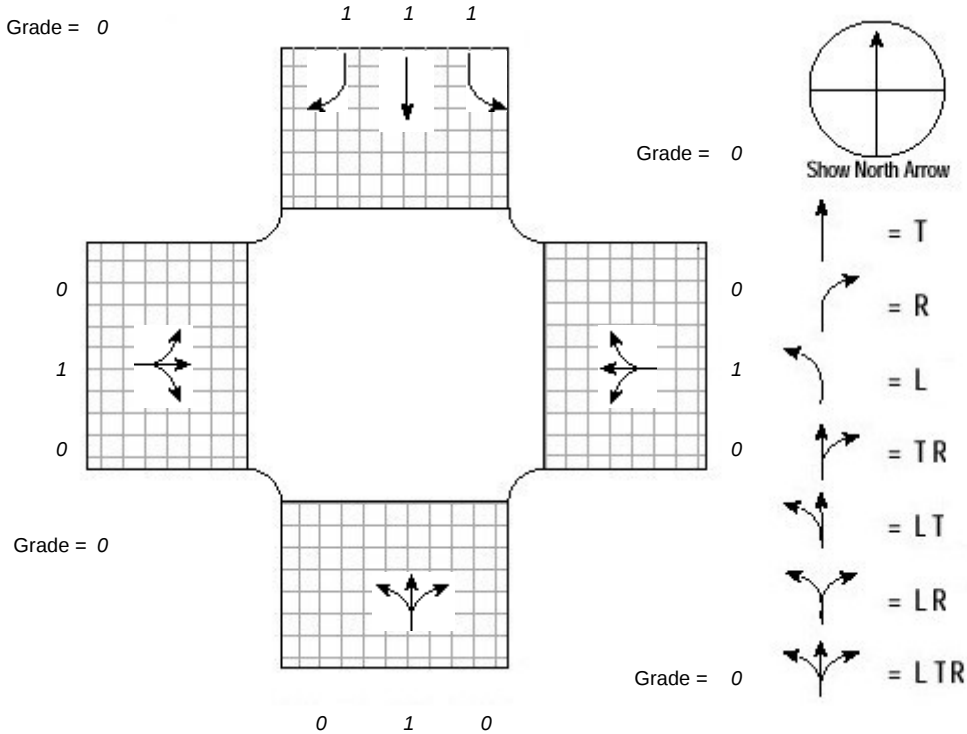
Lane Group Capacity, Control Delay, and LOS Determination

PF Factor	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
Control Delay	16.8	18.5		16.5	17.6	17.2	18.6	20.7		18.7	20.1	
Lane Group LOS	B	B		B	B	B	B	C		B	C	
Approach Delay	18.4			17.3			20.3			19.8		
Approach LOS	B			B			C			B		
Intersection Delay	19.1			Intersection LOS						B		

FULL REPORT

General Information				Site Information	
Analyst	LSB			Intersection	SW 184 Street/SW 157 Avenue
Agency or Co.	Cathy Sweetapple & Associates			Area Type	All other areas
Date Performed	08/20/2008			Jurisdiction	Miami-Dade
Time Period	PM Peak Hour			Analysis Year	Future without Project

Intersection Geometry



Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume (vph)	120	360	0	0	171	107	0	1	4	106	2	178
% Heavy Veh	2	2	2	2	2	2	2	2	2	2	2	2
PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Actuated (P/A)	P	P	P	P	P	P	A	A	A	A	A	A
Startup Lost Time		2.0			2.0			2.0		2.0	2.0	2.0
Extension of Effective Green		2.0			2.0			2.0		2.0	2.0	2.0
Arrival Type		3			3			3		3	3	3
Unit Extension		3.0			3.0			3.0		3.0	3.0	3.0
Ped/Bike/RTOR Volume	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0			12.0			12.0		12.0	12.0	12.0
Parking (Y or N)	N		N	N		N	N		N	N		N
Parking/Hour												
Bus Stops/Hour		0			0			0		0	0	0

Pedestrian Timing			3.2		3.2		3.2		3.2	
	EW Perm	02	03	04	NS Perm	06	07	08		
Timing	G = 60.0	G =	G =	G =	G = 40.0	G =	G =	G =		
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =		
Duration of Analysis (hrs) = 0.25					Cycle Length C = 110.0					

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	120	360	0	0	171	107	0	1	4	106	2	178
PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adjusted Flow Rate	126	379	0	0	180	113	0	1	4	112	2	187
Lane Group		LTR			LTR			LTR		L	T	R
Adjusted Flow Rate		505			293			5		112	2	187
Proportion of LT or RT	0.250	--	0.000	0.000	--	0.386	0.000	--	0.800	0.000	--	0.000
Saturation Flow Rate												
Base Satflow		1900			1900			1900		1900	1900	1900
Number of Lanes	0	1	0	0	1	0	0	1	0	1	1	1
f_w		1.000			1.000			1.000		1.000	1.000	1.000
f_{HV}		0.980			0.980			0.980		0.980	0.980	0.980
f_g		1.000			1.000			1.000		1.000	1.000	1.000
f_p		1.000			1.000			1.000		1.000	1.000	1.000
f_{bb}		1.000			1.000			1.000		1.000	1.000	1.000
f_a		1.000			1.000			1.000		1.000	1.000	1.000
f_{LU}		1.000			1.000			1.000		1.000	1.000	1.000
f_{LT}		0.785	--		1.000	--		1.000	--	0.754	1.000	--
Secondary f_{LT}			--			--			--			--

f_{RT}	--	1.000		--	0.948		--	0.892		--	1.000	0.850
f_{Lpb}		1.000	--		1.000	--		1.000	--	1.000	1.000	--
f_{Rpb}	--	1.000		--	1.000		--	1.000		--	1.000	1.000
Adjusted Satflow		1463			1766			1662		1405	1863	1583
Secondary Adjusted Satflow			--			--			--			--

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane Group		<i>LTR</i>			<i>LTR</i>			<i>LTR</i>		<i>L</i>	<i>T</i>	<i>R</i>
Adjusted Flow Rate		505			293			5		112	2	187
Satflow Rate		1463			1766			1662		1405	1863	1583
Lost Time		2.0			2.0			2.0		2.0	2.0	2.0
Green Ratio		0.55			0.55			0.36		0.36	0.36	0.36
Lane Group Capacity		798			963			604		511	677	576
v/c Ratio		0.63			0.30			0.01		0.22	0.00	0.32
Flow Ratio		0.35			0.17			0.00		0.08	0.00	0.12
Critical Lane Group		Y			N			N		N	N	Y
Sum Flow Ratios	0.46											
Lost Time/Cycle	10.00											
Critical v/c Ratio	0.51											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane Group		<i>LTR</i>			<i>LTR</i>			<i>LTR</i>		<i>L</i>	<i>T</i>	<i>R</i>
Adjusted Flow Rate		505			293			5		112	2	187
Lane Group Capacity		798			963			604		511	677	576
v/c Ratio		0.63			0.30			0.01		0.22	0.00	0.32
Green Ratio		0.55			0.55			0.36		0.36	0.36	0.36
Uniform Delay d ₁		17.4			13.6			22.3		24.2	22.3	25.3
Delay Factor k		0.50			0.50			0.11		0.11	0.11	0.11
Incremental Delay d ₂		3.8			0.8			0.0		0.2	0.0	0.3
PF Factor		1.000			1.000			1.000		1.000	1.000	1.000

Control Delay		21.2			14.4			22.3		24.4	22.3	25.6
Lane Group LOS		C			B			C		C	C	C
Approach Delay	21.2			14.4			22.3			25.1		
Approach LOS	C			B			C			C		
Intersection Delay	20.5			Intersection LOS						C		

Future with Project

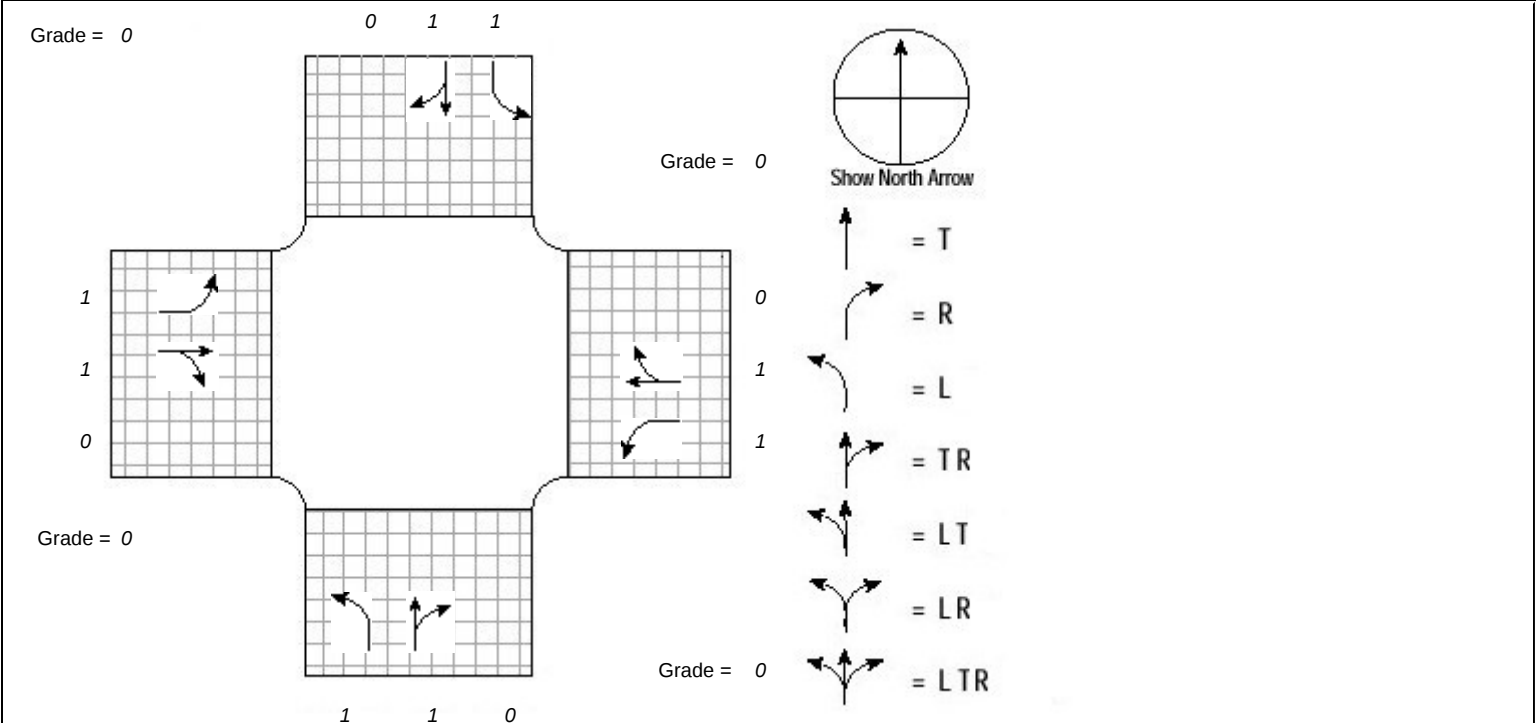
FULL REPORT	
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General Information	
1. Name of the Project:	...
2. Location:	...
3. Date:	...
4. Project Manager:	...
5. Client:	...
6. Budget:	...
7. Status:	...
8. Contact Information:	...
9. Other:	...

Analyst	LSB
Agency or Co.	Cathy Sweetapple & Associates
Date Performed	08/16/2008
Time Period	PM Peak Hour

Site Information

Intersection	SW 216 Street/SW 177 Avenue
Area Type	All other areas
Jurisdiction	Miami-Dade
Analysis Year	Future With Project

[illegible]

Volume and Timing Input	
Volume	100
Timing	100

[illegible]

Pedestrian Timing			3.2		3.2		3.2		3.2	
	EW Perm	02	03	04	Excl. Left	NS Perm	07	08		
Timing	G = 15.0	G =	G =	G =	G = 7.0	G = 41.0	G =	G =		
	Y = 5	Y =	Y =	Y =	Y = 3	Y = 5	Y =	Y =		
Duration of Analysis (hrs) = 0.25					Cycle Length C = 76.0					

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	52	93	81	48	95	86	31	713	51	90	837	61
PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adjusted Flow Rate	55	98	85	51	100	91	33	751	54	95	881	64
Lane Group	L	TR		L	TR		L	TR		L	TR	
Adjusted Flow Rate	55	183		51	191		33	805		95	945	
Proportion of LT or RT	0.000	--	0.464	0.000	--	0.476	0.000	--	0.067	0.000	--	0.068
Saturation Flow Rate												
Base Satflow	1900	1900		1900	1900		1900	1900		1900	1900	
Number of Lanes	1	1	0	1	1	0	1	1	0	1	1	0
f_w	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_{HV}	0.980	0.980		0.980	0.980		0.980	0.929		0.980	0.929	
f_g	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_p	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_{bb}	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_a	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_{LU}	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_{LT}	0.542	1.000	--	0.561	1.000	--	0.950	1.000	--	0.950	1.000	--
Secondary f_{LT}			--			--	0.091		--	0.129		--

f_{RT}	--	0.930		--	0.929		--	0.990		--	0.990	
f_{Lpb}	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--
f_{Rpb}	--	1.000		--	1.000		--	1.000		--	1.000	
Adjusted Satflow	1010	1733		1044	1730		1770	1748		1770	1748	
Secondary Adjusted Satflow			--			--	169		--	240		--

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>	
Adjusted Flow Rate	55	183		51	191		33	805		95	945	
Satflow Rate	1010	1733		1044	1730		1770	1748		1770	1748	
Lost Time	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Green Ratio	0.20	0.20		0.20	0.20		0.67	0.54		0.67	0.54	
Lane Group Capacity	199	342		206	341		261	943		302	943	
v/c Ratio	0.28	0.54		0.25	0.56		0.13	0.85		0.31	1.00	
Flow Ratio	0.05	0.11		0.05	0.11		0.02	0.46		0.05	0.54	
Critical Lane Group	<i>N</i>	<i>N</i>		<i>N</i>	<i>Y</i>		<i>N</i>	<i>N</i>		<i>Y</i>	<i>Y</i>	
Sum Flow Ratios	0.70											
Lost Time/Cycle	15.00											
Critical v/c Ratio	0.88											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>	
Adjusted Flow Rate	55	183		51	191		33	805		95	945	
Lane Group Capacity	199	342		206	341		261	943		302	943	
v/c Ratio	0.28	0.54		0.25	0.56		0.13	0.85		0.31	1.00	
Green Ratio	0.20	0.20		0.20	0.20		0.67	0.54		0.67	0.54	
Uniform Delay d ₁	25.9	27.4		25.7	27.5		14.3	14.9		10.5	17.5	
Delay Factor k	0.11	0.14		0.11	0.16		0.11	0.50		0.11	0.50	
Incremental Delay d ₂	0.8	1.7		0.6	2.1		0.2	9.7		0.6	29.8	
PF Factor	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	

Control Delay	26.7	29.0		26.4	29.6		14.5	24.6		11.1	47.3	
Lane Group LOS	C	C		C	C		B	C		B	D	
Approach Delay	28.5			28.9			24.2			44.0		
Approach LOS	C			C			C			D		
Intersection Delay	33.9			Intersection LOS						C		

FULL REPORT

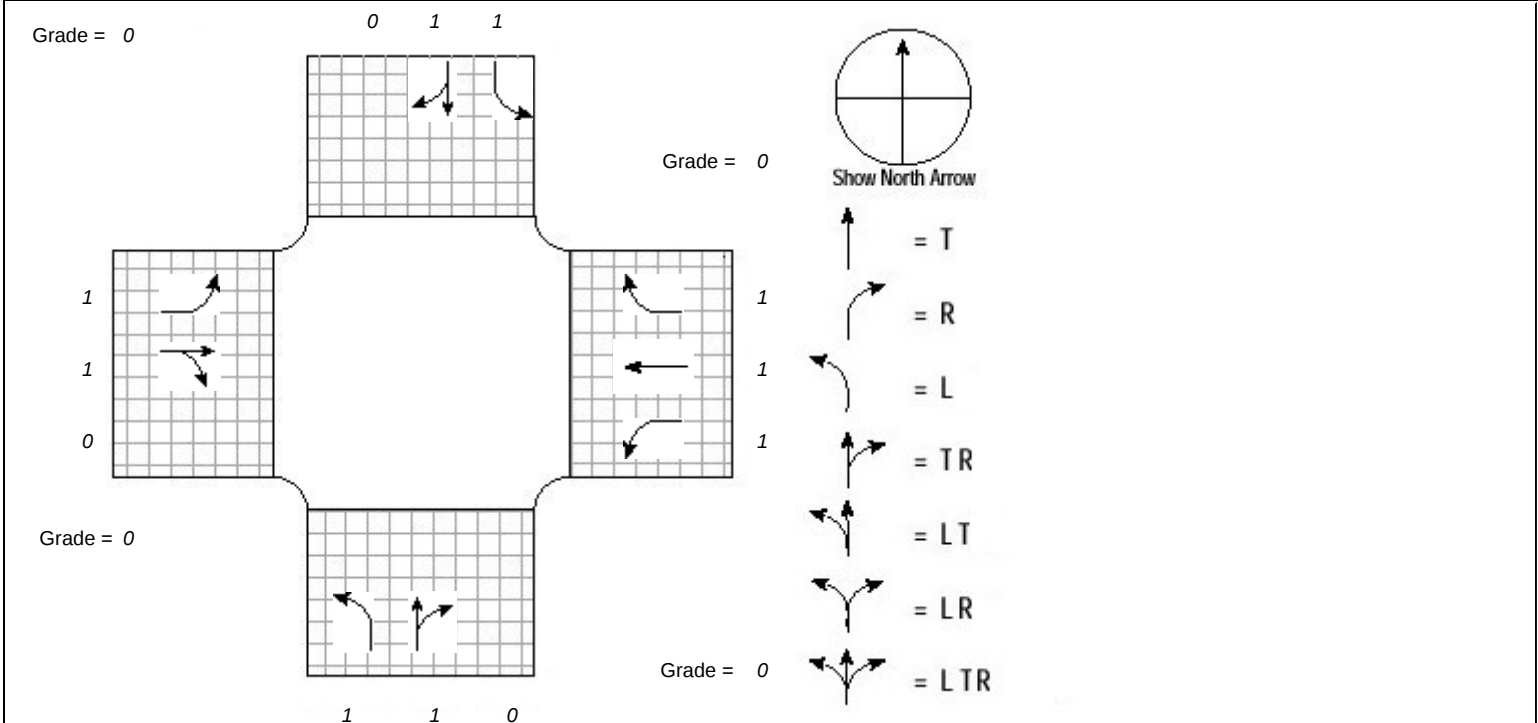
General Information	
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Analyst	LSB
Agency or Co.	Cathy Sweetapple & Associates
Date Performed	08/16/2008
Time Period	PM Peak Hour

Site Information

Intersection	SW 200 Street/SW 177 Avenue
Area Type	All other areas
Jurisdiction	Miami-Dade
Analysis Year	Future With Project

Intersection Geometry



Volume and Timing Input

[illegible]

Pedestrian Timing			3.2		3.2		3.2		3.2	
	EW Perm	02	03	04	Excl. Left	NS Perm	07	08		
Timing	G = 16.0	G =	G =	G =	G = 7.0	G = 43.0	G =	G =		
	Y = 5	Y =	Y =	Y =	Y = 3	Y = 5	Y =	Y =		
Duration of Analysis (hrs) = 0.25					Cycle Length C = 79.0					

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	83	110	26	89	126	168	69	728	106	156	804	48
PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adjusted Flow Rate	87	116	27	94	133	177	73	766	112	164	846	51
Lane Group	L	TR		L	T	R	L	TR		L	TR	
Adjusted Flow Rate	87	143		94	133	177	73	878		164	897	
Proportion of LT or RT	0.000	--	0.189	0.000	--	0.000	0.000	--	0.128	0.000	--	0.057
Saturation Flow Rate												
Base Satflow	1900	1900		1900	1900	1900	1900	1900		1900	1900	
Number of Lanes	1	1	0	1	1	1	1	1	0	1	1	0
f_w	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
f_{HV}	0.980	0.980		0.980	0.980	0.980	0.980	0.933		0.980	0.929	
f_g	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
f_p	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
f_{bb}	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
f_a	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
f_{LU}	1.000	1.000		1.000	1.000	1.000	1.000	1.000		1.000	1.000	
f_{LT}	0.671	1.000	--	0.648	1.000	--	0.950	1.000	--	0.950	1.000	--
Secondary f_{LT}			--			--	0.087		--	0.087		--

f_{RT}	--	0.972		--	1.000	0.850	--	0.981		--	0.991	
f_{Lpb}	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--
f_{Rpb}	--	1.000		--	1.000	1.000	--	1.000		--	1.000	
Adjusted Satflow	1250	1810		1206	1863	1583	1770	1738		1770	1750	
Secondary Adjusted Satflow			--			--	162		--	162		--

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>TR</i>		<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>	
Adjusted Flow Rate	<i>87</i>	<i>143</i>		<i>94</i>	<i>133</i>	<i>177</i>	<i>73</i>	<i>878</i>		<i>164</i>	<i>897</i>	
Satflow Rate	<i>1250</i>	<i>1810</i>		<i>1206</i>	<i>1863</i>	<i>1583</i>	<i>1770</i>	<i>1738</i>		<i>1770</i>	<i>1750</i>	
Lost Time	<i>2.0</i>	<i>2.0</i>		<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	<i>2.0</i>		<i>2.0</i>	<i>2.0</i>	
Green Ratio	<i>0.20</i>	<i>0.20</i>		<i>0.20</i>	<i>0.20</i>	<i>0.20</i>	<i>0.67</i>	<i>0.54</i>		<i>0.67</i>	<i>0.54</i>	
Lane Group Capacity	<i>253</i>	<i>367</i>		<i>244</i>	<i>377</i>	<i>321</i>	<i>251</i>	<i>946</i>		<i>251</i>	<i>953</i>	
v/c Ratio	<i>0.34</i>	<i>0.39</i>		<i>0.39</i>	<i>0.35</i>	<i>0.55</i>	<i>0.29</i>	<i>0.93</i>		<i>0.65</i>	<i>0.94</i>	
Flow Ratio	<i>0.07</i>	<i>0.08</i>		<i>0.08</i>	<i>0.07</i>	<i>0.11</i>	<i>0.04</i>	<i>0.51</i>		<i>0.09</i>	<i>0.51</i>	
Critical Lane Group	<i>N</i>	<i>N</i>		<i>N</i>	<i>N</i>	<i>Y</i>	<i>N</i>	<i>N</i>		<i>Y</i>	<i>Y</i>	
Sum Flow Ratios	<i>0.71</i>											
Lost Time/Cycle	<i>15.00</i>											
Critical v/c Ratio	<i>0.88</i>											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>TR</i>		<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>	
Adjusted Flow Rate	<i>87</i>	<i>143</i>		<i>94</i>	<i>133</i>	<i>177</i>	<i>73</i>	<i>878</i>		<i>164</i>	<i>897</i>	
Lane Group Capacity	<i>253</i>	<i>367</i>		<i>244</i>	<i>377</i>	<i>321</i>	<i>251</i>	<i>946</i>		<i>251</i>	<i>953</i>	
v/c Ratio	<i>0.34</i>	<i>0.39</i>		<i>0.39</i>	<i>0.35</i>	<i>0.55</i>	<i>0.29</i>	<i>0.93</i>		<i>0.65</i>	<i>0.94</i>	
Green Ratio	<i>0.20</i>	<i>0.20</i>		<i>0.20</i>	<i>0.20</i>	<i>0.20</i>	<i>0.67</i>	<i>0.54</i>		<i>0.67</i>	<i>0.54</i>	
Uniform Delay d ₁	<i>27.0</i>	<i>27.3</i>		<i>27.2</i>	<i>27.1</i>	<i>28.3</i>	<i>13.5</i>	<i>16.6</i>		<i>14.9</i>	<i>16.8</i>	
Delay Factor k	<i>0.11</i>	<i>0.11</i>		<i>0.11</i>	<i>0.11</i>	<i>0.15</i>	<i>0.11</i>	<i>0.50</i>		<i>0.23</i>	<i>0.50</i>	
Incremental Delay d ₂	<i>0.8</i>	<i>0.7</i>		<i>1.0</i>	<i>0.6</i>	<i>2.0</i>	<i>0.6</i>	<i>16.3</i>		<i>6.0</i>	<i>18.0</i>	
PF Factor	<i>1.000</i>	<i>1.000</i>		<i>1.000</i>	<i>1.000</i>	<i>1.000</i>	<i>1.000</i>	<i>1.000</i>		<i>1.000</i>	<i>1.000</i>	

Control Delay	27.8	28.0		28.3	27.6	30.3	14.1	32.9		20.9	34.8	
Lane Group LOS	C	C		C	C	C	B	C		C	C	
Approach Delay	27.9			29.0			31.5			32.7		
Approach LOS	C			C			C			C		
Intersection Delay	31.3			Intersection LOS						C		

FULL REPORT	
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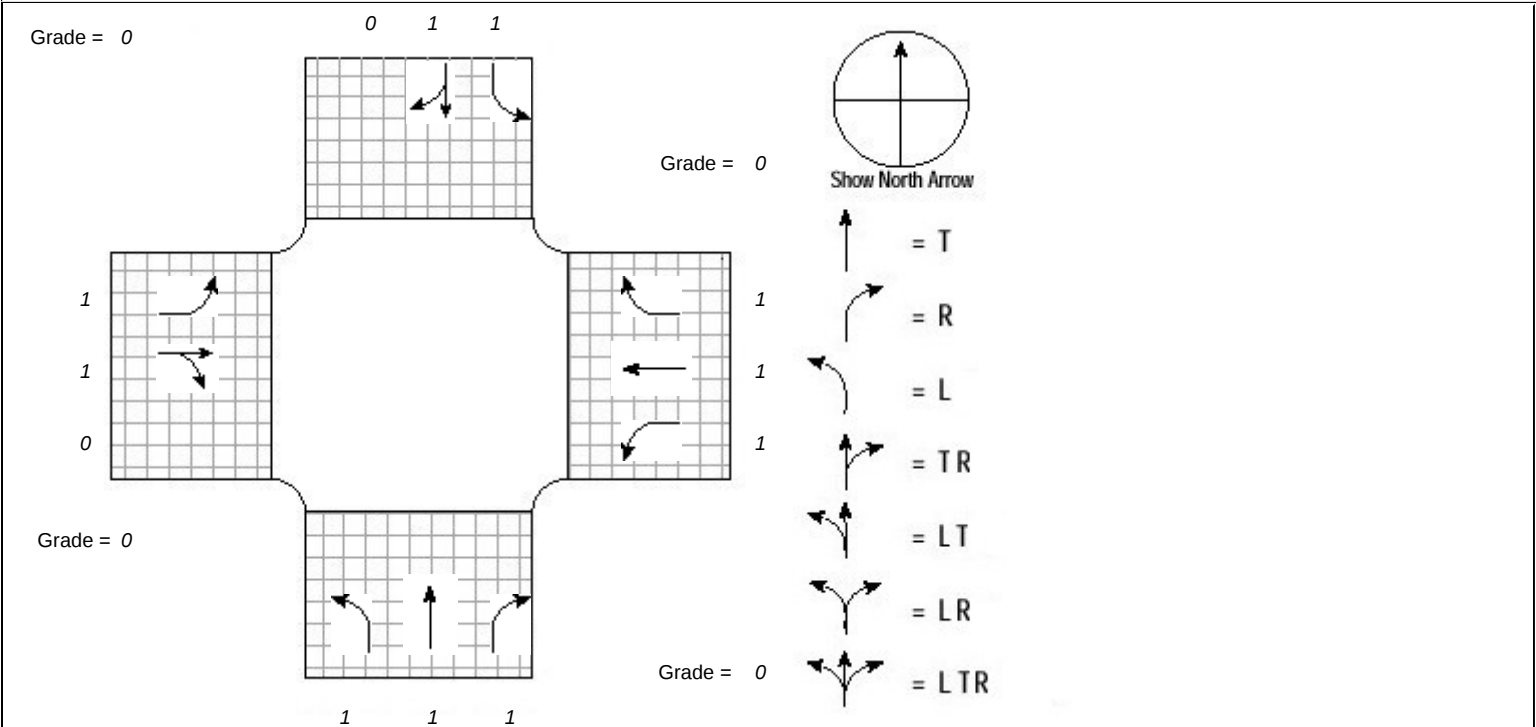
General Information	
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Analyst	LSB
Agency or Co.	Cathy Sweetapple & Associates
Date Performed	08/16/2008
Time Period	PM Peak Hour

Site Information

Intersection	SW 184 Street/Krome Avenue
Area Type	All other areas
Jurisdiction	Miami-Dade
Analysis Year	Future with Project

Intersection Geometry	
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Volume and Timing Input	
Volume	Timing
1000	1000
2000	2000
3000	3000
4000	4000
5000	5000
6000	6000
7000	7000
8000	8000
9000	9000
10000	10000

[illegible]

Pedestrian Timing			3.2		3.2		3.2		3.2	
	EW Perm	02	03	04	Excl. Left	NS Perm	07	08		
Timing	G = 21.0	G =	G =	G =	G = 7.0	G = 53.0	G =	G =		
	Y = 5	Y =	Y =	Y =	Y = 3	Y = 5	Y =	Y =		
Duration of Analysis (hrs) = 0.25					Cycle Length C = 94.0					

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	67	33	8	230	27	145	3	795	218	203	890	28
PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adjusted Flow Rate	71	35	8	242	28	153	3	837	229	214	937	29
Lane Group	L	TR		L	T	R	L	T	R	L	TR	
Adjusted Flow Rate	71	43		242	28	153	3	837	229	214	966	
Proportion of LT or RT	0.000	--	0.186	0.000	--	0.000	0.000	--	0.000	0.000	--	0.030
Saturation Flow Rate												
Base Satflow	1900	1900		1900	1900	1900	1900	1900	1900	1900	1900	
Number of Lanes	1	1	0	1	1	1	1	1	1	1	1	0
f_w	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_{HV}	0.980	0.980		0.980	0.980	0.980	0.980	0.926	0.980	0.980	0.927	
f_g	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_p	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_{bb}	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_a	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_{LU}	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_{LT}	0.739	1.000	--	0.729	1.000	--	0.950	1.000	--	0.950	1.000	--
Secondary f_{LT}			--			--	0.071		--	0.128		--

f_{RT}	--	0.972		--	1.000	0.850	--	1.000	0.850	--	0.995	
f_{Lpb}	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--
f_{Rpb}	--	1.000		--	1.000	1.000	--	1.000	1.000	--	1.000	
Adjusted Satflow	1377	1811		1358	1863	1583	1770	1759	1583	1770	1754	
Secondary Adjusted Satflow			--			--	133		--	238		--

General Information

Capacity Analysis

Lane Group Capacity, Control Delay, and LOS Determination

Control Delay	30.3	29.2		48.5	28.8	32.2	17.2	25.8	11.1	29.4	43.4	
Lane Group LOS	C	C		D	C	C	B	C	B	C	D	
Approach Delay	29.8			41.3			22.6			40.9		
Approach LOS	C			D			C			D		
Intersection Delay	33.5			Intersection LOS						C		

FULL REPORT

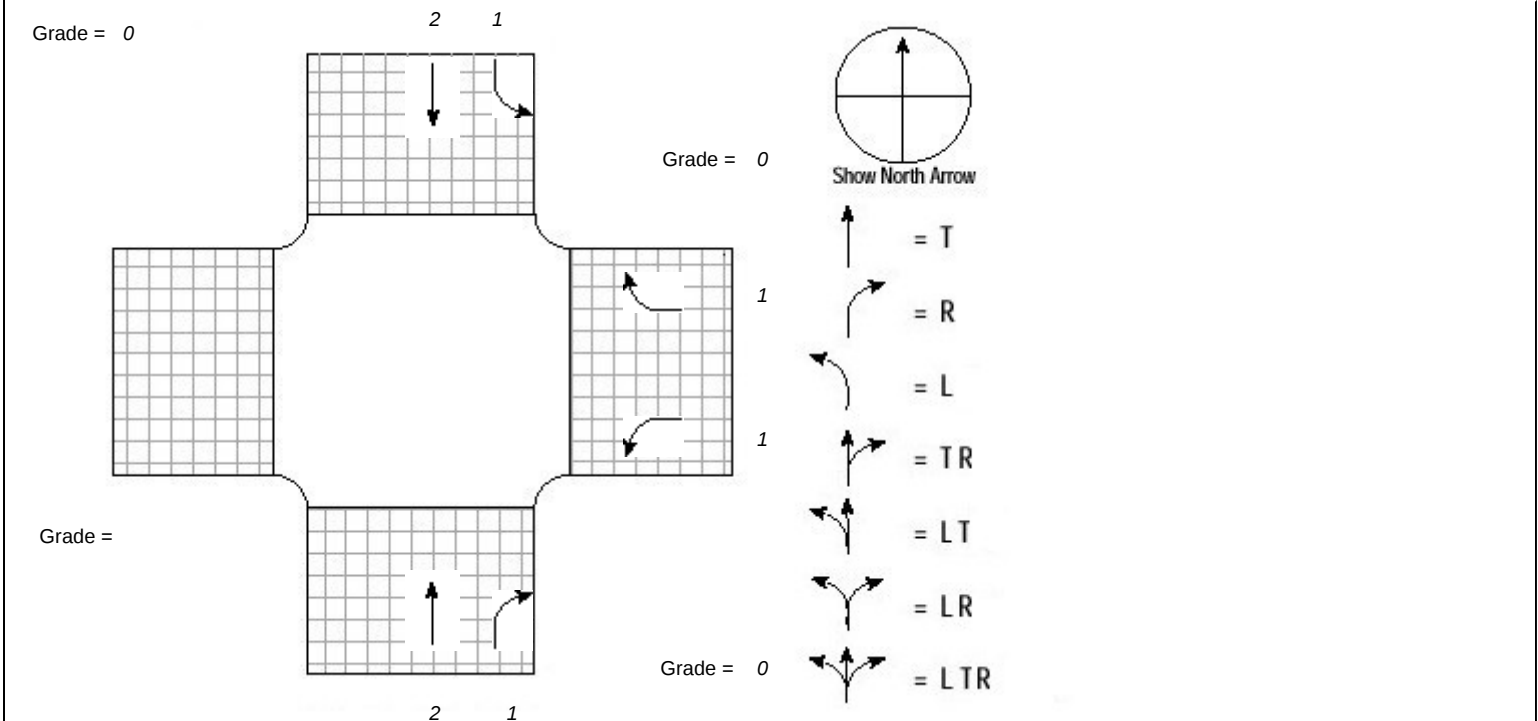
General Information	
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Analyst	LSB
Agency or Co.	Cathy Sweetapple & Associates
Date Performed	08/20/2008
Time Period	PM Peak Hour

Site Information

Intersection	SW 152 Street/SW 177 Avenue
Area Type	All other areas
Jurisdiction	Miami-Dade
Analysis Year	Future with Project

Intersection Geometry	
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Volume and Timing Input	
Volume	1000
Timing	1000

[illegible]

Pedestrian Timing			3.2			3.2		3.2	
	WB Only	02	03	04	SB Only	NS Perm	07	08	
Timing	G = 15.0	G =	G =	G =	G = 10.0	G = 46.0	G =	G =	
	Y = 5	Y =	Y =	Y =	Y = 4	Y = 5	Y =	Y =	
Duration of Analysis (hrs) = 0.25					Cycle Length C = 85.0				

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume				85		85		894	114	129	1036	
PHF				0.95		0.95		0.95	0.95	0.95	0.95	
Adjusted Flow Rate				89		89		941	15	136	1091	
Lane Group				L		R		T	R	L	T	
Adjusted Flow Rate				89		89		941	15	136	1091	
Proportion of LT or RT		--			--		0.000	--	0.000	0.000	--	0.000
Saturation Flow Rate												
Base Satflow				1900		1900		1900	1900	1900	1900	
Number of Lanes				1		1		2	1	1	2	
f_w				1.000		1.000		1.000	1.000	1.000	1.000	
f_{HV}				0.980		0.980		0.926	0.980	0.980	0.926	
f_g				1.000		1.000		1.000	1.000	1.000	1.000	
f_p				1.000		1.000		1.000	1.000	1.000	1.000	
f_{bb}				1.000		1.000		1.000	1.000	1.000	1.000	
f_a				1.000		1.000		1.000	1.000	1.000	1.000	
f_{LU}				1.000		1.000		0.952	1.000	1.000	0.952	
f_{LT}			--	0.950		--		1.000	--	0.950	1.000	--
Secondary f_{LT}			--			--			--	0.215		--

f_{RT}	--			--		0.850	--	1.000	0.850	--	1.000	
f_{Lpb}			--	1.000		--		1.000	--	1.000	1.000	--
f_{Rpb}	--			--		1.000	--	1.000	1.000	--	1.000	
Adjusted Satflow				1770		1583		3350	1583	1770	3350	
Secondary Adjusted Satflow			--			--			--	400		--

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane Group				<i>L</i>		<i>R</i>		<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	
Adjusted Flow Rate				89		89		941	15	136	1091	
Satflow Rate				1770		1583		3350	1583	1770	3350	
Lost Time				2.0		2.0		2.0	2.0	2.0	2.0	
Green Ratio				0.18		0.18		0.54	0.54	0.71	0.71	
Lane Group Capacity				312		279		1813	857	443	2365	
v/c Ratio				0.29		0.32		0.52	0.02	0.31	0.46	
Flow Ratio				0.05		0.06		0.28	0.01	0.08	0.33	
Critical Lane Group				<i>N</i>		<i>Y</i>		<i>Y</i>	<i>N</i>	<i>Y</i>	<i>N</i>	
Sum Flow Ratios	0.41											
Lost Time/Cycle	15.00											
Critical v/c Ratio	0.50											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane Group				<i>L</i>		<i>R</i>		<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	
Adjusted Flow Rate				89		89		941	15	136	1091	
Lane Group Capacity				312		279		1813	857	443	2365	
v/c Ratio				0.29		0.32		0.52	0.02	0.31	0.46	
Green Ratio				0.18		0.18		0.54	0.54	0.71	0.71	
Uniform Delay d ₁				30.4		30.5		12.4	9.0	5.7	5.5	
Delay Factor k				0.11		0.11		0.50	0.50	0.50	0.50	
				0.5		0.7		1.1	0.0	1.8	0.7	

Incremental Delay d ₂												
PF Factor				1.000		1.000		1.000	1.000	1.000	1.000	
Control Delay				30.9		31.2		13.5	9.1	7.5	6.1	
Lane Group LOS				C		C		B	A	A	A	
Approach Delay				31.0			13.4			6.3		
Approach LOS				C			B			A		
Intersection Delay	11.0			Intersection LOS						B		

FULL REPORT	
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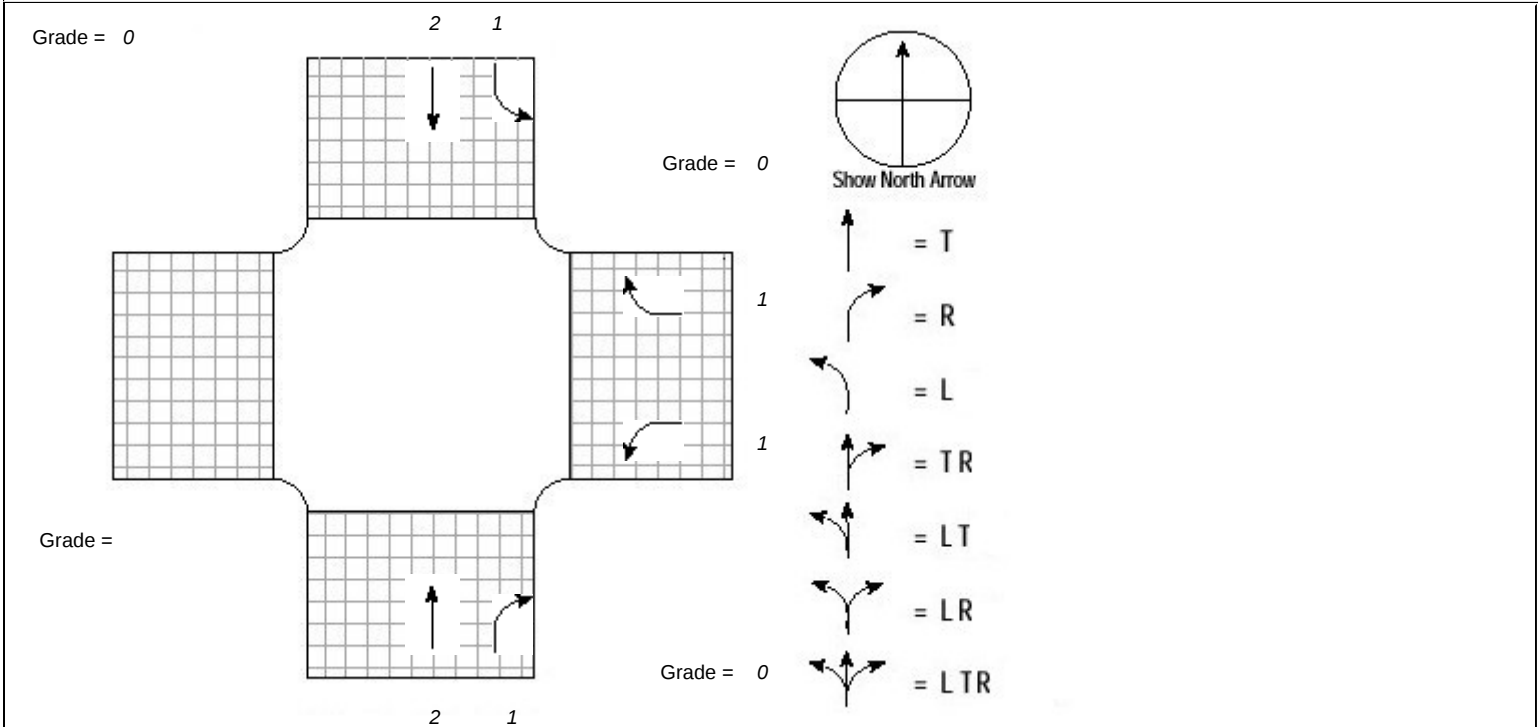
General Information	
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Analyst	LSB
Agency or Co.	Cathy Sweetapple & Associates
Date Performed	08/20/2008
Time Period	PM Peak Hour

Site Information

Intersection	SW 136 Street/SW 177 Avenue
Area Type	All other areas
Jurisdiction	Miami-Dade
Analysis Year	Future with Project

Intersection Geometry



Volume and Timing Input	
Volume	Timing
1000	1000
2000	2000
3000	3000
4000	4000
5000	5000
6000	6000
7000	7000
8000	8000
9000	9000
10000	10000

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume (vph)				0		170		979	0	243	1165	
% Heavy Veh				2		2		8	2	2	8	
PHF				0.95		0.95		0.95	0.95	0.95	0.95	
Actuated (P/A)				A		A		P	P	P	P	
Startup Lost Time				2.0		2.0		2.0	2.0	2.0	2.0	
Extension of Effective Green				2.0		2.0		2.0	2.0	2.0	2.0	
Arrival Type				3		3		3	3	3	3	
Unit Extension				3.0		3.0		3.0	3.0	3.0	3.0	
Ped/Bike/RTOR Volume				0	0	0	0	0	100	0	0	
Lane Width				12.0		12.0		12.0	12.0	12.0	12.0	
Parking (Y or N)				N		N	N		N	N		N
Parking/Hour												
Bus Stops/Hour				0		0		0	0	0	0	

Pedestrian Timing			3.2			3.2		3.2	
	WB Only	02	03	04	SB Only	NS Perm	07	08	
Timing	G = 20.0	G =	G =	G =	G = 10.0	G = 41.0	G =	G =	
	Y = 5	Y =	Y =	Y =	Y = 4	Y = 5	Y =	Y =	
Duration of Analysis (hrs) = 0.25					Cycle Length C = 85.0				

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET													
General Information													
Project Description <i>Parkland DRI</i>													
Volume Adjustment													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Volume				0		170		979	0	243	1165		
PHF				0.95		0.95		0.95	0.95	0.95	0.95		
Adjusted Flow Rate				0		179		1031	0	256	1226		
Lane Group				L		R		T	R	L	T		
Adjusted Flow Rate				0		179		1031	0	256	1226		
Proportion of LT or RT		--			--		0.000	--	0.000	0.000	--	0.000	
Saturation Flow Rate													
Base Satflow				1900		1900		1900	1900	1900	1900		
Number of Lanes				1		1		2	1	1	2		
f_w				1.000		1.000		1.000	1.000	1.000	1.000		
f_{HV}				0.980		0.980		0.926	0.980	0.980	0.926		
f_g				1.000		1.000		1.000	1.000	1.000	1.000		
f_p				1.000		1.000		1.000	1.000	1.000	1.000		
f_{bb}				1.000		1.000		1.000	1.000	1.000	1.000		
f_a				1.000		1.000		1.000	1.000	1.000	1.000		
f_{LU}				1.000		1.000		0.952	1.000	1.000	0.952		
f_{LT}			--	0.950		--		1.000	--	0.950	1.000	--	
Secondary f_{LT}			--			--			--	0.161		--	

f_{RT}	--			--		0.850	--	1.000	0.850	--	1.000	
f_{Lpb}			--	1.000		--		1.000	--	1.000	1.000	--
f_{Rpb}	--			--		1.000	--	1.000	1.000	--	1.000	
Adjusted Satflow				1770		1583		3350	1583	1770	3350	
Secondary Adjusted Satflow			--			--			--	301		--

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane Group				<i>L</i>		<i>R</i>		<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	
Adjusted Flow Rate				<i>0</i>		<i>179</i>		<i>1031</i>	<i>0</i>	<i>256</i>	<i>1226</i>	
Satflow Rate				<i>1770</i>		<i>1583</i>		<i>3350</i>	<i>1583</i>	<i>1770</i>	<i>3350</i>	
Lost Time				<i>2.0</i>		<i>2.0</i>		<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	
Green Ratio				<i>0.24</i>		<i>0.24</i>		<i>0.48</i>	<i>0.48</i>	<i>0.65</i>	<i>0.65</i>	
Lane Group Capacity				<i>416</i>		<i>372</i>		<i>1616</i>	<i>764</i>	<i>367</i>	<i>2168</i>	
v/c Ratio				<i>0.00</i>		<i>0.48</i>		<i>0.64</i>	<i>0.00</i>	<i>0.70</i>	<i>0.57</i>	
Flow Ratio				<i>0.00</i>		<i>0.11</i>		<i>0.31</i>	<i>0.00</i>	<i>0.12</i>	<i>0.37</i>	
Critical Lane Group				<i>N</i>		<i>Y</i>		<i>Y</i>	<i>N</i>	<i>Y</i>	<i>N</i>	
Sum Flow Ratios	<i>0.54</i>											
Lost Time/Cycle	<i>15.00</i>											
Critical v/c Ratio	<i>0.65</i>											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane Group				<i>L</i>		<i>R</i>		<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	
Adjusted Flow Rate				<i>0</i>		<i>179</i>		<i>1031</i>	<i>0</i>	<i>256</i>	<i>1226</i>	
Lane Group Capacity				<i>416</i>		<i>372</i>		<i>1616</i>	<i>764</i>	<i>367</i>	<i>2168</i>	
v/c Ratio				<i>0.00</i>		<i>0.48</i>		<i>0.64</i>	<i>0.00</i>	<i>0.70</i>	<i>0.57</i>	
Green Ratio				<i>0.24</i>		<i>0.24</i>		<i>0.48</i>	<i>0.48</i>	<i>0.65</i>	<i>0.65</i>	
Uniform Delay d ₁				<i>24.9</i>		<i>28.0</i>		<i>16.5</i>	<i>11.4</i>	<i>10.3</i>	<i>8.3</i>	
Delay Factor k				<i>0.11</i>		<i>0.11</i>		<i>0.50</i>	<i>0.50</i>	<i>0.50</i>	<i>0.50</i>	
				<i>0.0</i>		<i>1.0</i>		<i>1.9</i>	<i>0.0</i>	<i>10.5</i>	<i>1.1</i>	

Incremental Delay d ₂												
PF Factor				1.000		1.000		1.000	1.000	1.000	1.000	
Control Delay				24.9		29.0		18.4	11.4	20.8	9.4	
Lane Group LOS				C		C		B	B	C	A	
Approach Delay				29.0			18.4			11.4		
Approach LOS				C			B			B		
Intersection Delay	15.2			Intersection LOS						B		

Pedestrian Timing			3.2		3.2		3.2		3.2	
	WB Only	EW Perm	03	04	SB Only	NS Perm	07	08		
Timing	G = 15.0	G = 20.0	G =	G =	G = 20.0	G = 38.0	G =	G =		
	Y = 4	Y = 5	Y =	Y =	Y = 3	Y = 5	Y =	Y =		
Duration of Analysis (hrs) = 0.25							Cycle Length C = 110.0			

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	12	13	5	458	3	237	3	649	522	436	865	13
PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adjusted Flow Rate	13	14	5	482	3	249	3	683	444	459	911	14
Lane Group		LTR		L	T	R	L	T	R	L	TR	
Adjusted Flow Rate		32		482	3	249	3	683	444	459	925	
Proportion of LT or RT	0.406	--	0.156	0.000	--	0.000	0.000	--	0.000	0.000	--	0.015
Saturation Flow Rate												
Base Satflow		1900		1900	1900	1900	1900	1900	1900	1900	1900	
Number of Lanes	0	1	0	1	1	1	1	2	1	1	2	0
f_w		1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_{HV}		0.980		0.980	0.980	0.980	0.980	0.926	0.980	0.980	0.927	
f_g		1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_p		1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_{bb}		1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_a		1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_{LU}		1.000		1.000	1.000	1.000	1.000	0.952	1.000	1.000	0.952	
f_{LT}		0.918	--	0.950	1.000	--	0.309	1.000	--	0.950	1.000	--
Secondary f_{LT}			--	0.720	1.000	--			--	0.221		--

f_{RT}	--	0.979		--	1.000	0.850	--	1.000	0.850	--	0.998	
f_{Lpb}		1.000	--	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--
f_{Rpb}	--	1.000		--	1.000	1.000	--	1.000	1.000	--	1.000	
Adjusted Satflow		1674		1770	1863	1583	576	3350	1583	1770	3345	
Secondary Adjusted Satflow			--	1340	1863	--			--	412		--

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane Group		<i>LTR</i>		<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>TR</i>	
Adjusted Flow Rate		<i>32</i>		<i>482</i>	<i>3</i>	<i>249</i>	<i>3</i>	<i>683</i>	<i>444</i>	<i>459</i>	<i>925</i>	
Satflow Rate		<i>1674</i>		<i>1770</i>	<i>1863</i>	<i>1583</i>	<i>576</i>	<i>3350</i>	<i>1583</i>	<i>1770</i>	<i>3345</i>	
Lost Time		<i>2.0</i>		<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	
Green Ratio		<i>0.18</i>		<i>0.35</i>	<i>0.35</i>	<i>0.35</i>	<i>0.35</i>	<i>0.35</i>	<i>0.35</i>	<i>0.55</i>	<i>0.55</i>	
Lane Group Capacity		<i>304</i>		<i>533</i>	<i>661</i>	<i>561</i>	<i>199</i>	<i>1157</i>	<i>547</i>	<i>476</i>	<i>1855</i>	
v/c Ratio		<i>0.11</i>		<i>0.90</i>	<i>0.00</i>	<i>0.44</i>	<i>0.02</i>	<i>0.59</i>	<i>0.81</i>	<i>0.96</i>	<i>0.50</i>	
Flow Ratio		<i>0.02</i>		<i>0.14</i>	<i>0.00</i>	<i>0.16</i>	<i>0.01</i>	<i>0.20</i>	<i>0.28</i>	<i>0.18</i>	<i>0.28</i>	
Critical Lane Group		<i>N</i>		<i>Y</i>	<i>N</i>	<i>N</i>	<i>N</i>	<i>N</i>	<i>N</i>	<i>Y</i>	<i>N</i>	
Sum Flow Ratios	<i>0.83</i>											
Lost Time/Cycle	<i>10.00</i>											
Critical v/c Ratio	<i>0.91</i>											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane Group		<i>LTR</i>		<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>TR</i>	
Adjusted Flow Rate		<i>32</i>		<i>482</i>	<i>3</i>	<i>249</i>	<i>3</i>	<i>683</i>	<i>444</i>	<i>459</i>	<i>925</i>	
Lane Group Capacity		<i>304</i>		<i>533</i>	<i>661</i>	<i>561</i>	<i>199</i>	<i>1157</i>	<i>547</i>	<i>476</i>	<i>1855</i>	
v/c Ratio		<i>0.11</i>		<i>0.90</i>	<i>0.00</i>	<i>0.44</i>	<i>0.02</i>	<i>0.59</i>	<i>0.81</i>	<i>0.96</i>	<i>0.50</i>	
Green Ratio		<i>0.18</i>		<i>0.35</i>	<i>0.35</i>	<i>0.35</i>	<i>0.35</i>	<i>0.35</i>	<i>0.35</i>	<i>0.55</i>	<i>0.55</i>	
Uniform Delay d ₁		<i>37.5</i>		<i>34.3</i>	<i>23.0</i>	<i>27.2</i>	<i>23.7</i>	<i>29.6</i>	<i>32.7</i>	<i>21.1</i>	<i>15.1</i>	
Delay Factor k		<i>0.11</i>		<i>0.43</i>	<i>0.11</i>	<i>0.11</i>	<i>0.11</i>	<i>0.18</i>	<i>0.35</i>	<i>0.47</i>	<i>0.11</i>	
Incremental Delay d ₂		<i>0.2</i>		<i>18.9</i>	<i>0.0</i>	<i>0.6</i>	<i>0.0</i>	<i>0.8</i>	<i>9.1</i>	<i>32.1</i>	<i>0.2</i>	

PF Factor		1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
Control Delay		37.7		53.1	23.0	27.8	23.7	30.4	41.8	53.2	15.3	
Lane Group LOS		D		D	C	C	C	C	D	D	B	
Approach Delay	37.7			44.4			34.9			27.9		
Approach LOS	D			D			C			C		
Intersection Delay	34.1			Intersection LOS						C		

FULL REPORT

General Information				Site Information	
Analyst	LSB			Intersection	SW 136 Street/SW 157 Avenue
Agency or Co.	Cathy Sweetapple & Associates			Area Type	All other areas
Date Performed	08/20/2008			Jurisdiction	Miami-Dade
Time Period	PM Peak Hour			Analysis Year	Future with Project

Intersection Geometry

Grade =

Grade = 0



= T

= R

= L

= TR

= LT

= LR

= LTR

Grade = 0

Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume (vph)		490	57	503	559		222		219			
% Heavy Veh		2	2	2	2		2		2			
PHF		0.95	0.95	0.95	0.95		0.95		0.95			
Actuated (P/A)		P	P	A	P		A		A			
Startup Lost Time		2.0	2.0	2.0	2.0		2.0		2.0			
Extension of Effective Green		2.0	2.0	2.0	2.0		2.0		2.0			
Arrival Type		3	3	3	3		3		3			
Unit Extension		3.0	3.0	3.0	3.0		3.0		3.0			
Ped/Bike/RTOR Volume	0	0	0	0	0		0	0	0			
Lane Width		12.0	12.0	12.0	12.0		12.0		12.0			
Parking (Y or N)	N		N	N		N	N		N			
Parking/Hour												
Bus Stops/Hour		0	0	0	0		0		0			

Pedestrian Timing			3.2		3.2		3.2		
	WB Only	EW Perm	03	04	NB Only	06	07	08	
Timing	G = 15.0	G = 36.0	G =	G =	G = 20.0	G =	G =	G =	
	Y = 4	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =	
Duration of Analysis (hrs) = 0.25						Cycle Length C = 85.0			

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume		490	57	503	559		222		219			
PHF		0.95	0.95	0.95	0.95		0.95		0.95			
Adjusted Flow Rate		516	60	529	588		234		231			
Lane Group		T	R	L	T		L		R			
Adjusted Flow Rate		516	60	529	588		234		231			
Proportion of LT or RT	0.000	--	0.000	0.000	--	0.000		--			--	
Saturation Flow Rate												
Base Satflow		1900	1900	1900	1900		1900		1900			
Number of Lanes		2	1	1	2		1		2			
f_w		1.000	1.000	1.000	1.000		1.000		1.000			
f_{HV}		0.980	0.980	0.980	0.980		0.980		0.980			
f_g		1.000	1.000	1.000	1.000		1.000		1.000			
f_p		1.000	1.000	1.000	1.000		1.000		1.000			
f_{bb}		1.000	1.000	1.000	1.000		1.000		1.000			
f_a		1.000	1.000	1.000	1.000		1.000		1.000			
f_{LU}		0.952	1.000	1.000	0.952		1.000		0.885			
f_{LT}		1.000	--	0.950	1.000	--	0.950		--			--
Secondary f_{LT}			--	0.362		--			--			--

f_{RT}	--	1.000	0.850	--	1.000		--		0.850	--		
f_{Lpb}		1.000	--	1.000	1.000	--	1.000		--			--
f_{Rpb}	--	1.000	1.000	--	1.000		--		1.000	--		
Adjusted Satflow		3547	1583	1770	3547		1770		2803			
Secondary Adjusted Satflow			--	674		--			--			--

General Information

Capacity Analysis

Lane Group Capacity, Control Delay, and LOS Determination

PF Factor		1.000	1.000	1.000	1.000		1.000		1.000			
Control Delay		17.2	14.9	18.7	17.7		30.4		27.4			
Lane Group LOS		B	B	B	B		C		C			
Approach Delay	16.9			18.2			28.9					
Approach LOS	B			B			C					
Intersection Delay	20.2			Intersection LOS						C		

FULL REPORT

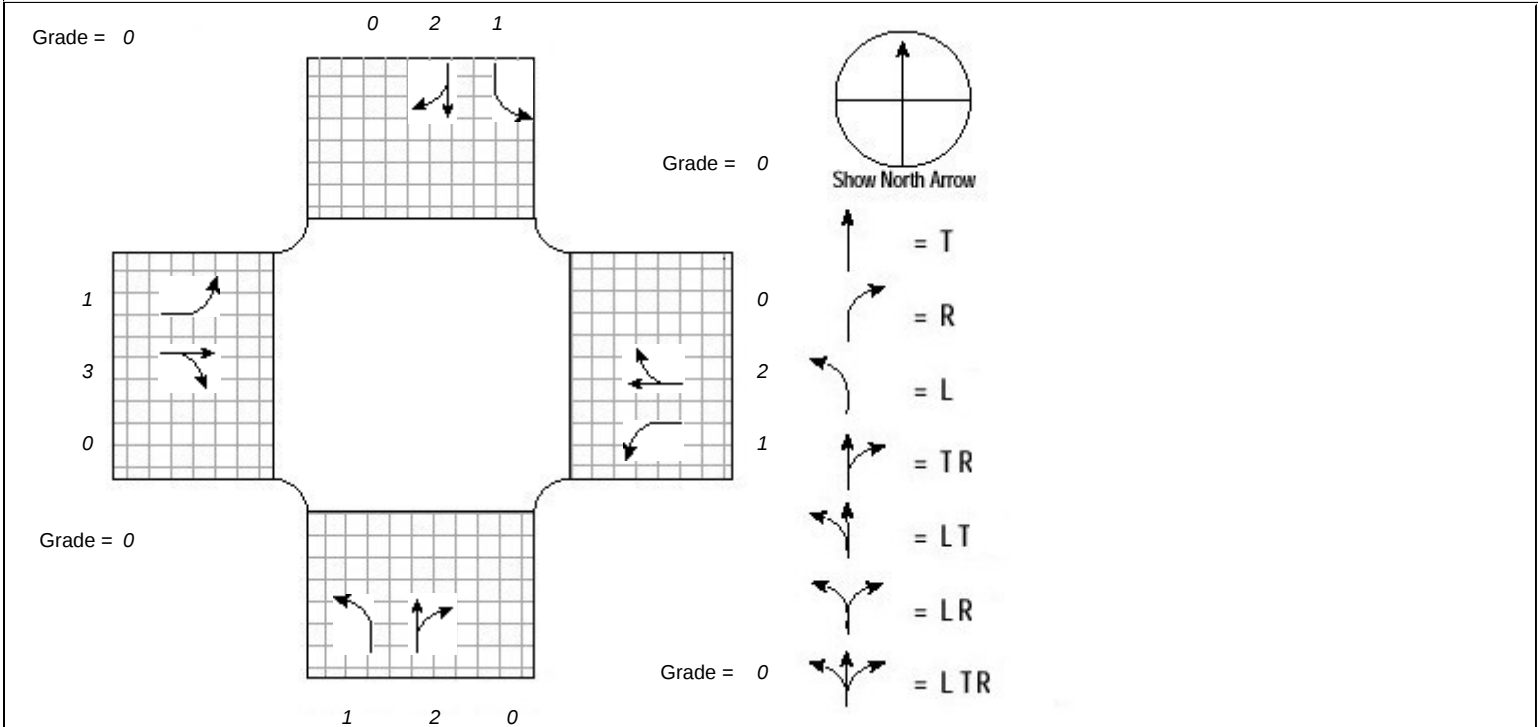
General Information	
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Analyst	LSB
Agency or Co.	Cathy Sweetapple & Associates
Date Performed	08/20/2008
Time Period	PM Peak Hour

Site Information

Intersection	SW 152 Street/SW 157 Avenue
Area Type	All other areas
Jurisdiction	Miami-Dade
Analysis Year	Future with Project

Intersection Geometry



Volume and Timing Input

[illegible]

Pedestrian Timing			3.2		3.2		3.2		3.2	
	Excl. Left	EW Perm	03	04	Excl. Left	NS Perm	07	08		
Timing	G = 10.0	G = 39.0	G =	G =	G = 21.0	G = 22.0	G =	G =		
	Y = 4	Y = 5	Y =	Y =	Y = 4	Y = 5	Y =	Y =		
Duration of Analysis (hrs) = 0.25							Cycle Length C = 110.0			

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	199	837	267	76	970	183	422	99	93	242	207	230
PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adjusted Flow Rate	209	881	281	80	1021	193	444	104	98	255	218	242
Lane Group	L	TR		L	TR		L	TR		L	TR	
Adjusted Flow Rate	209	1162		80	1214		444	202		255	460	
Proportion of LT or RT	0.000	--	0.242	0.000	--	0.159	0.000	--	0.485	0.000	--	0.526
Saturation Flow Rate												
Base Satflow	1900	1900		1900	1900		1900	1900		1900	1900	
Number of Lanes	1	3	0	1	2	0	1	2	0	1	2	0
f_w	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_{HV}	0.980	0.980		0.980	0.980		0.980	0.980		0.980	0.980	
f_g	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_p	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_{bb}	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_a	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_{LU}	1.000	0.908		1.000	0.952		1.000	0.952		1.000	0.952	
f_{LT}	0.950	1.000	--	0.950	1.000	--	0.950	1.000	--	0.950	1.000	--
Secondary f_{LT}	0.093	0.093	--	0.113		--	0.232		--	0.493		--

f_{RT}	--	0.964		--	0.976		--	0.927		--	0.921	
f_{Lpb}	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--
f_{Rpb}	--	1.000		--	1.000		--	1.000		--	1.000	
Adjusted Satflow	1770	4890		1770	3462		1770	3289		1770	3267	
Secondary Adjusted Satflow	173	455	--	210		--	432		--	918		--

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>	
Adjusted Flow Rate	209	1162		80	1214		444	202		255	460	
Satflow Rate	1770	4890		1770	3462		1770	3289		1770	3267	
Lost Time	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	
Green Ratio	0.48	0.35		0.48	0.35		0.43	0.20		0.43	0.20	
Lane Group Capacity	229	1734		243	1227		440	658		555	653	
v/c Ratio	0.91	0.67		0.33	0.99		1.01	0.31		0.46	0.70	
Flow Ratio	0.09	0.24		0.05	0.35		0.19	0.06		0.14	0.14	
Critical Lane Group	Y	N		N	Y		Y	N		N	N	
Sum Flow Ratios	0.88											
Lost Time/Cycle	15.00											
Critical v/c Ratio	1.02											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>		<i>L</i>	<i>TR</i>	
Adjusted Flow Rate	209	1162		80	1214		444	202		255	460	
Lane Group Capacity	229	1734		243	1227		440	658		555	653	
v/c Ratio	0.91	0.67		0.33	0.99		1.01	0.31		0.46	0.70	
Green Ratio	0.48	0.35		0.48	0.35		0.43	0.20		0.43	0.20	
Uniform Delay d ₁	28.9	30.1		18.2	35.3		26.0	37.5		21.3	41.0	
Delay Factor k	0.43	0.50		0.11	0.50		0.50	0.11		0.11	0.27	
	36.7	2.1		0.8	23.3		45.2	0.3		0.6	3.5	

Incremental Delay d_2												
PF Factor	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
Control Delay	65.6	32.1		19.0	58.6		71.2	37.8		21.9	44.4	
Lane Group LOS	E	C		B	E		E	D		C	D	
Approach Delay	37.2			56.1			60.7			36.4		
Approach LOS	D			E			E			D		
Intersection Delay	46.9			Intersection LOS						D		

FULL REPORT

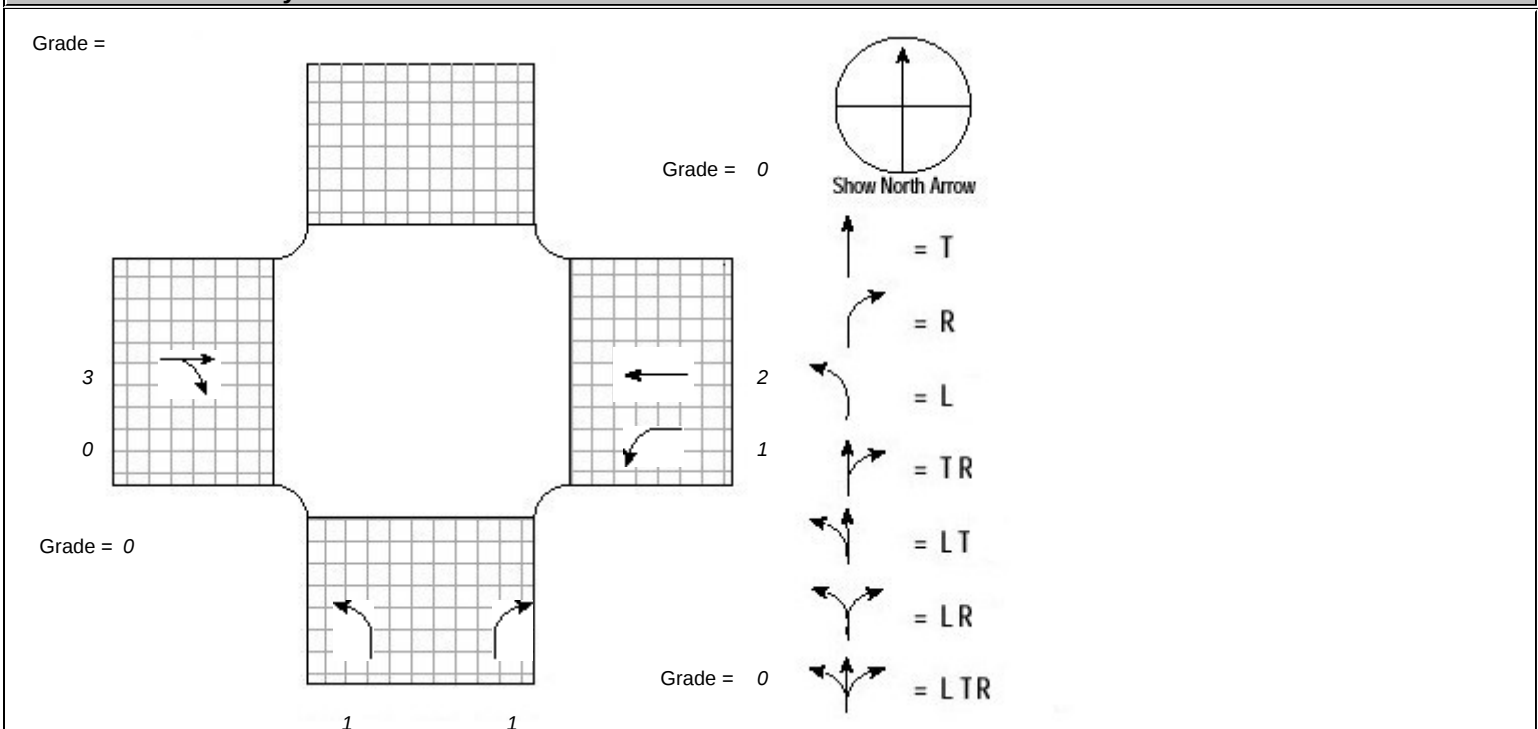
General Information	
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Analyst	LSB
Agency or Co.	Cathy Sweetapple & Associates
Date Performed	08/20/2008
Time Period	PM Peak Hour

Site Information

Intersection	SW 152 Street/SW 147 Avenue
Area Type	All other areas
Jurisdiction	Miami-Dade
Analysis Year	Future with Project

Intersection Geometry	
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Volume and Timing Input

[illegible]

Pedestrian Timing			3.2		3.2		3.2			
	WB Only	EW Perm	03	04	NB Only	06	07	08		
Timing	G = 15.0	G = 39.0	G =	G =	G = 18.0	G =	G =	G =		
	Y = 3	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =		
Duration of Analysis (hrs) = 0.25					Cycle Length C = 85.0					

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume		1289	52	476	1646		42		345			
PHF		0.95	0.95	0.95	0.95		0.95		0.95			
Adjusted Flow Rate		1357	55	501	1733		44		258			
Lane Group		TR		L	T		L		R			
Adjusted Flow Rate		1412		501	1733		44		258			
Proportion of LT or RT	0.000	--	0.039	0.000	--	0.000		--			--	
Saturation Flow Rate												
Base Satflow		1900		1900	1900		1900		1900			
Number of Lanes		3	0	1	2		1		1			
f_w		1.000		1.000	1.000		1.000		1.000			
f_{HV}		0.980		0.980	0.980		0.980		0.980			
f_g		1.000		1.000	1.000		1.000		1.000			
f_p		1.000		1.000	1.000		1.000		1.000			
f_{bb}		1.000		1.000	1.000		1.000		1.000			
f_a		1.000		1.000	1.000		1.000		1.000			
f_{LU}		0.908		1.000	0.952		1.000		1.000			
f_{LT}		1.000	--	0.950	1.000	--	0.950		--			--
Secondary f_{LT}			--	0.099	0.734	--			--			--

f_{RT}	--	0.994		--	1.000		--		0.850	--		
f_{Lpb}		1.000	--	1.000	1.000	--	1.000		--			--
f_{Rpb}	--	1.000		--	1.000		--		1.000	--		
Adjusted Satflow		5044		1770	3547		1770		1583			
Secondary Adjusted Satflow			--	184	2602	--			--			--

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane Group		<i>TR</i>		<i>L</i>	<i>T</i>		<i>L</i>		<i>R</i>			
Adjusted Flow Rate		<i>1412</i>		<i>501</i>	<i>1733</i>		<i>44</i>		<i>258</i>			
Satflow Rate		<i>5044</i>		<i>1770</i>	<i>3547</i>		<i>1770</i>		<i>1583</i>			
Lost Time		<i>2.0</i>		<i>2.0</i>	<i>2.0</i>		<i>2.0</i>		<i>2.0</i>			
Green Ratio		<i>0.46</i>		<i>0.67</i>	<i>0.67</i>		<i>0.21</i>		<i>0.21</i>			
Lane Group Capacity		<i>2314</i>		<i>403</i>	<i>2379</i>		<i>375</i>		<i>335</i>			
v/c Ratio		<i>0.61</i>		<i>1.24</i>	<i>0.73</i>		<i>0.12</i>		<i>0.77</i>			
Flow Ratio		<i>0.28</i>		<i>0.18</i>	<i>0.49</i>		<i>0.02</i>		<i>0.16</i>			
Critical Lane Group		<i>N</i>		<i>Y</i>	<i>N</i>		<i>N</i>		<i>Y</i>			
Sum Flow Ratios	<i>1.37</i>											
Lost Time/Cycle	<i>10.00</i>											
Critical v/c Ratio	<i>1.55</i>											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane Group		<i>TR</i>		<i>L</i>	<i>T</i>		<i>L</i>		<i>R</i>			
Adjusted Flow Rate		<i>1412</i>		<i>501</i>	<i>1733</i>		<i>44</i>		<i>258</i>			
Lane Group Capacity		<i>2314</i>		<i>403</i>	<i>2379</i>		<i>375</i>		<i>335</i>			
v/c Ratio		<i>0.61</i>		<i>1.24</i>	<i>0.73</i>		<i>0.12</i>		<i>0.77</i>			
Green Ratio		<i>0.46</i>		<i>0.67</i>	<i>0.67</i>		<i>0.21</i>		<i>0.21</i>			
Uniform Delay d ₁		<i>17.3</i>		<i>24.3</i>	<i>9.0</i>		<i>27.1</i>		<i>31.6</i>			
Delay Factor k		<i>0.50</i>		<i>0.50</i>	<i>0.50</i>		<i>0.11</i>		<i>0.32</i>			
		<i>1.2</i>		<i>128.8</i>	<i>2.0</i>		<i>0.1</i>		<i>10.5</i>			

Incremental Delay d_2												
PF Factor		1.000		1.000	1.000		1.000		1.000			
Control Delay		18.5		153.2	11.0		27.2		42.0			
Lane Group LOS		B		F	B		C		D			
Approach Delay	18.5			42.9			39.9					
Approach LOS	B			D			D					
Intersection Delay	33.9			Intersection LOS						C		

FULL REPORT

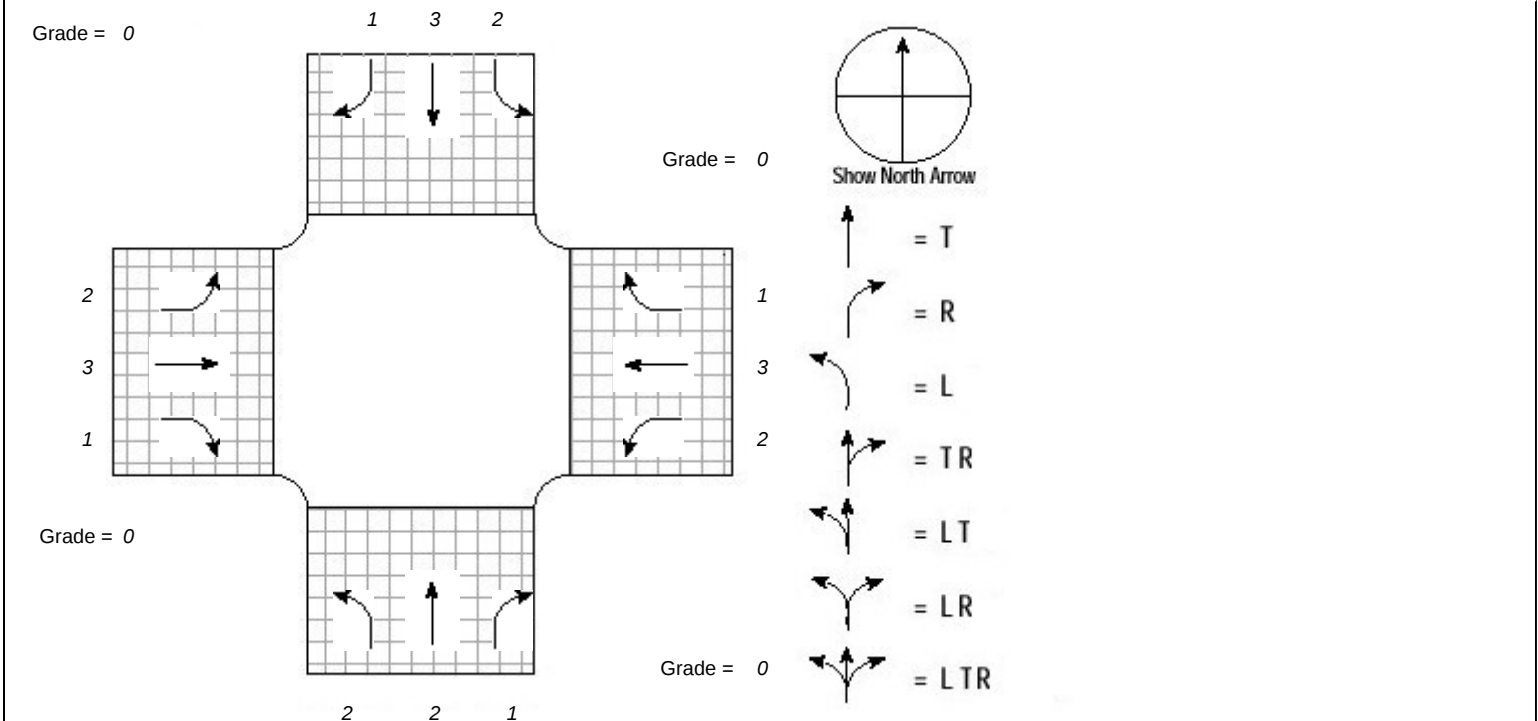
General Information	
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Analyst	LSB
Agency or Co.	Cathy Sweetapple & Associates
Date Performed	08/22/2008
Time Period	PM Peak Hour

Site Information

Intersection	SW 152 Street/SW 137 Avenue
Area Type	All other areas
Jurisdiction	Miami-Dade
Analysis Year	Future with Project

Intersection Geometry	
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Volume and Timing Input

[illegible]

Pedestrian Timing			3.2		3.2		3.2		3.2	
	Excl. Left	WB Only	Thru & RT	04	Excl. Left	SB Only	Thru & RT	08		
Timing	G = 13.0	G = 8.0	G = 40.0	G =	G = 15.0	G = 6.0	G = 30.0	G =		
	Y = 4	Y = 4	Y = 6	Y =	Y = 4	Y = 4	Y = 6	Y =		
Duration of Analysis (hrs) = 0.25					Cycle Length C = 140.0					

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	339	1249	247	669	1909	785	440	932	445	597	1245	524
PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adjusted Flow Rate	357	1315	260	704	2009	826	463	981	468	628	1311	552
Lane Group	L	T	R	L	T	R	L	T	R	L	T	R
Adjusted Flow Rate	357	1315	260	704	2009	826	463	981	468	628	1311	552
Proportion of LT or RT	0.000	--	0.000	0.000	--	0.000	0.000	--	0.000	0.000	--	0.000
Saturation Flow Rate												
Base Satflow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Number of Lanes	2	3	1	2	3	1	2	2	1	2	3	1
f_w	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_{HV}	0.980	0.980	0.980	0.980	0.980	0.980	0.980	0.980	0.980	0.980	0.980	0.980
f_g	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_p	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_{bb}	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_a	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_{LU}	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
f_{LT}	0.950	1.000	--	0.950	1.000	--	0.950	1.000	--	0.950	1.000	--
Secondary f_{LT}			--			--			--			--

f_{RT}	--	1.000	0.850	--	1.000	0.850	--	1.000	0.850	--	1.000	0.850
f_{Lpb}	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--
f_{Rpb}	--	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--	1.000	1.000
Adjusted Satflow	3539	5588	1583	3539	5588	1583	3539	3725	1583	3539	5588	1583
Secondary Adjusted Satflow			--			--			--			--

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>
Adjusted Flow Rate	357	1315	260	704	2009	826	463	981	468	628	1311	552
Satflow Rate	3539	5588	1583	3539	5588	1583	3539	3725	1583	3539	5588	1583
Lost Time	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Green Ratio	0.09	0.29	0.44	0.18	0.37	0.59	0.11	0.21	0.44	0.18	0.29	0.42
Lane Group Capacity	329	1597	690	632	2076	938	379	798	690	632	1597	667
v/c Ratio	1.09	0.82	0.38	1.11	0.97	0.88	1.22	1.23	0.68	0.99	0.82	0.83
Flow Ratio	0.10	0.24	0.16	0.20	0.36	0.52	0.13	0.26	0.30	0.18	0.23	0.35
Critical Lane Group	Y	N	N	N	Y	N	N	Y	N	Y	N	N
Sum Flow Ratios	0.90											
Lost Time/Cycle	20.00											
Critical v/c Ratio	1.05											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>
Adjusted Flow Rate	357	1315	260	704	2009	826	463	981	468	628	1311	552
Lane Group Capacity	329	1597	690	632	2076	938	379	798	690	632	1597	667
v/c Ratio	1.09	0.82	0.38	1.11	0.97	0.88	1.22	1.23	0.68	0.99	0.82	0.83
Green Ratio	0.09	0.29	0.44	0.18	0.37	0.59	0.11	0.21	0.44	0.18	0.29	0.42
Uniform Delay d ₁	63.5	46.7	26.7	57.5	43.2	24.3	62.5	55.0	31.6	57.4	46.7	36.0
Delay Factor k	0.50	0.50	0.11	0.50	0.50	0.41	0.50	0.50	0.25	0.50	0.36	0.37
	74.3	4.9	0.3	71.3	13.5	9.7	121.3	114.1	2.7	34.1	3.6	8.5

Incremental Delay d_2												
PF Factor	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Control Delay	137.8	51.6	27.0	128.8	56.7	34.0	183.8	169.1	34.3	91.5	50.2	44.5
Lane Group LOS	F	D	C	F	E	C	F	F	C	F	D	D
Approach Delay	64.2			65.7			139.7			59.4		
Approach LOS	E			E			F			E		
Intersection Delay	78.2			Intersection LOS						E		

FULL REPORT

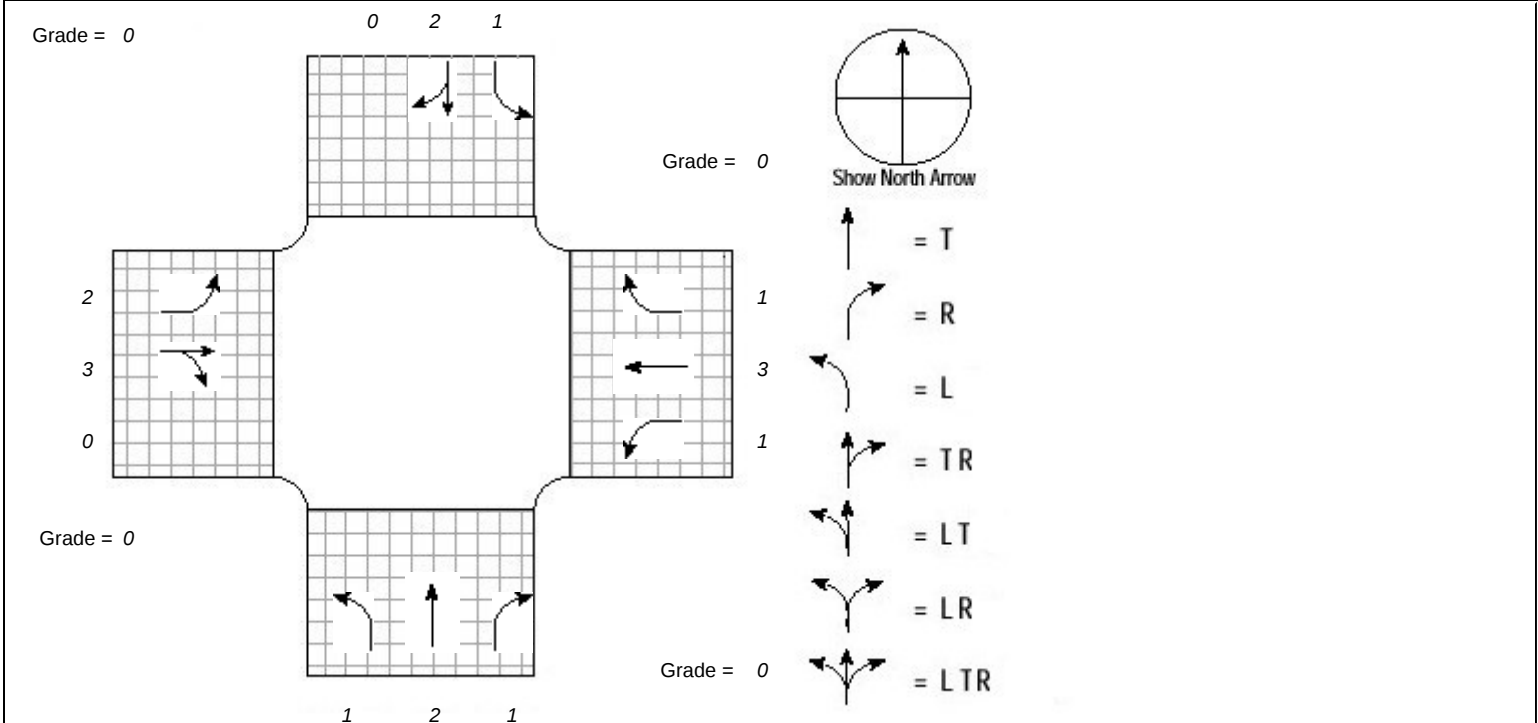
General Information	
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Analyst	LSB
Agency or Co.	Cathy Sweetapple & Associates
Date Performed	08/20/2008
Time Period	PM Peak Hour

Site Information

Intersection	SW 152 Street/SW 117 Avenue
Area Type	All other areas
Jurisdiction	Miami-Dade
Analysis Year	Future with Project

Intersection Geometry	
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Volume and Timing Input	
Volume	1000
Timing	1000

[illegible]

Pedestrian Timing			3.2		3.2		3.2		3.2	
	Excl. Left	EB Only	Thru & RT	04	Excl. Left	NB Only	Thru & RT	08		
Timing	G = 8.0	G = 15.0	G = 56.0	G =	G = 19.0	G = 10.0	G = 30.0	G =		
	Y = 3	Y = 3	Y = 5	Y =	Y = 3	Y = 3	Y = 5	Y =		
Duration of Analysis (hrs) = 0.25					Cycle Length C = 160.0					

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	520	1943	320	40	1897	19	346	337	82	87	483	100
PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adjusted Flow Rate	547	2045	337	42	1997	20	364	355	86	92	508	105
Lane Group	L	TR		L	T	R	L	T	R	L	TR	
Adjusted Flow Rate	547	2382		42	1997	20	364	355	86	92	613	
Proportion of LT or RT	0.000	--	0.141	0.000	--	0.000	0.000	--	0.000	0.000	--	0.171
Saturation Flow Rate												
Base Satflow	1900	1900		1900	1900	1900	1900	1900	1900	1900	1900	
Number of Lanes	2	3	0	1	3	1	1	2	1	1	2	0
f_w	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_{HV}	0.980	0.980		0.980	0.980	0.980	0.980	0.980	0.980	0.980	0.980	
f_g	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_p	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_{bb}	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_a	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_{LU}	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
f_{LT}	0.950	1.000	--	0.950	1.000	--	0.950	1.000	--	0.950	1.000	--
Secondary f_{LT}			--			--			--			--

f_{RT}	--	0.979		--	1.000	0.850	--	1.000	0.850	--	0.974	
f_{Lpb}	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--
f_{Rpb}	--	1.000		--	1.000	1.000	--	1.000	1.000	--	1.000	
Adjusted Satflow	3539	5470		1770	5588	1583	1770	3725	1583	1770	3630	
Secondary Adjusted Satflow			--			--			--			--

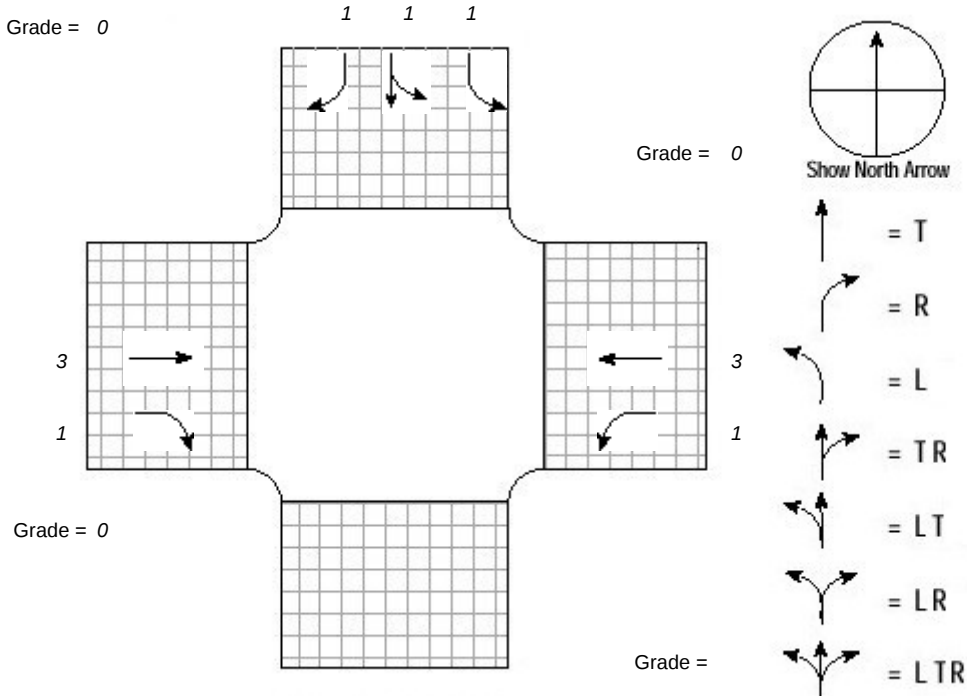
CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>TR</i>		<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>TR</i>	
Adjusted Flow Rate	<i>547</i>	<i>2382</i>		<i>42</i>	<i>1997</i>	<i>20</i>	<i>364</i>	<i>355</i>	<i>86</i>	<i>92</i>	<i>613</i>	
Satflow Rate	<i>3539</i>	<i>5470</i>		<i>1770</i>	<i>5588</i>	<i>1583</i>	<i>1770</i>	<i>3725</i>	<i>1583</i>	<i>1770</i>	<i>3630</i>	
Lost Time	<i>2.0</i>	<i>2.0</i>		<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	<i>2.0</i>	
Green Ratio	<i>0.16</i>	<i>0.46</i>		<i>0.05</i>	<i>0.35</i>	<i>0.35</i>	<i>0.20</i>	<i>0.27</i>	<i>0.27</i>	<i>0.12</i>	<i>0.19</i>	
Lane Group Capacity	<i>575</i>	<i>2530</i>		<i>89</i>	<i>1956</i>	<i>554</i>	<i>354</i>	<i>1001</i>	<i>425</i>	<i>210</i>	<i>681</i>	
v/c Ratio	<i>0.95</i>	<i>0.94</i>		<i>0.47</i>	<i>1.02</i>	<i>0.04</i>	<i>1.03</i>	<i>0.35</i>	<i>0.20</i>	<i>0.44</i>	<i>0.90</i>	
Flow Ratio	<i>0.15</i>	<i>0.44</i>		<i>0.02</i>	<i>0.36</i>	<i>0.01</i>	<i>0.21</i>	<i>0.10</i>	<i>0.05</i>	<i>0.05</i>	<i>0.17</i>	
Critical Lane Group	<i>Y</i>	<i>N</i>		<i>N</i>	<i>Y</i>	<i>N</i>	<i>Y</i>	<i>N</i>	<i>N</i>	<i>N</i>	<i>Y</i>	
Sum Flow Ratios	<i>0.89</i>											
Lost Time/Cycle	<i>16.00</i>											
Critical v/c Ratio	<i>0.98</i>											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>TR</i>		<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>	<i>R</i>	<i>L</i>	<i>TR</i>	
Adjusted Flow Rate	<i>547</i>	<i>2382</i>		<i>42</i>	<i>1997</i>	<i>20</i>	<i>364</i>	<i>355</i>	<i>86</i>	<i>92</i>	<i>613</i>	
Lane Group Capacity	<i>575</i>	<i>2530</i>		<i>89</i>	<i>1956</i>	<i>554</i>	<i>354</i>	<i>1001</i>	<i>425</i>	<i>210</i>	<i>681</i>	
v/c Ratio	<i>0.95</i>	<i>0.94</i>		<i>0.47</i>	<i>1.02</i>	<i>0.04</i>	<i>1.03</i>	<i>0.35</i>	<i>0.20</i>	<i>0.44</i>	<i>0.90</i>	
Green Ratio	<i>0.16</i>	<i>0.46</i>		<i>0.05</i>	<i>0.35</i>	<i>0.35</i>	<i>0.20</i>	<i>0.27</i>	<i>0.27</i>	<i>0.12</i>	<i>0.19</i>	
Uniform Delay d ₁	<i>66.4</i>	<i>40.9</i>		<i>73.9</i>	<i>52.0</i>	<i>34.2</i>	<i>64.0</i>	<i>47.3</i>	<i>45.2</i>	<i>65.5</i>	<i>63.5</i>	
Delay Factor k	<i>0.46</i>	<i>0.50</i>		<i>0.11</i>	<i>0.50</i>	<i>0.50</i>	<i>0.50</i>	<i>0.11</i>	<i>0.11</i>	<i>0.11</i>	<i>0.42</i>	
	<i>25.9</i>	<i>8.6</i>		<i>3.9</i>	<i>25.8</i>	<i>0.1</i>	<i>55.3</i>	<i>0.2</i>	<i>0.2</i>	<i>1.5</i>	<i>15.1</i>	

Incremental Delay d_2												
PF Factor	1.000	1.000		1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	
Control Delay	92.3	49.6		77.9	77.8	34.4	119.3	47.5	45.5	67.0	78.6	
Lane Group LOS	F	D		E	E	C	F	D	D	E	E	
Approach Delay	57.5			77.4			79.7			77.1		
Approach LOS	E			E			E			E		
Intersection Delay	68.7			Intersection LOS						E		

FULL REPORT

General Information				Site Information	
Analyst	LSB			Intersection	SW 152 Street/SR 821 W
Agency or Co.	Cathy Sweetapple & Associates			Area Type	All other areas
Date Performed	08/20/2008			Jurisdiction	Miami-Dade
Time Period	PM Peak Hour			Analysis Year	Future with Project

Intersection Geometry



Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume (vph)		1527	474	209	1485					421	107	476
% Heavy Veh		2	2	2	2					2	2	2
PHF		0.95	0.95	0.95	0.95					0.95	0.95	0.95
Actuated (P/A)		A	A	A	A					P	P	P
Startup Lost Time		2.0	2.0	2.0	2.0					2.0	2.0	2.0
Extension of Effective Green		2.0	2.0	2.0	2.0					2.0	2.0	2.0
Arrival Type		3	3	3	3					3	3	3
Unit Extension		3.0	3.0	3.0	3.0					3.0	3.0	3.0
Ped/Bike/RTOR Volume	0	0	0	0	0					0	0	100
Lane Width		12.0	12.0	12.0	12.0					12.0	12.0	12.0
Parking (Y or N)	N		N	N		N				N		N
Parking/Hour												
Bus Stops/Hour		0	0	0	0					0	0	0

Pedestrian Timing			3.2		3.2		3.2	
	WB Only	EW Perm	03	04	SB Only	06	07	08
Timing	G = 11.0	G = 50.0	G =	G =	G = 35.0	G =	G =	G =
	Y = 3	Y = 5	Y =	Y =	Y = 6	Y =	Y =	Y =
Duration of Analysis (hrs) = 0.25					Cycle Length C = 110.0			

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET

General Information

Project Description *Parkland DRI*

Volume Adjustment

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume		1527	474	209	1485					421	107	476
PHF		0.95	0.95	0.95	0.95					0.95	0.95	0.95
Adjusted Flow Rate		1607	499	220	1563					443	113	396
Lane Group		T	R	L	T					L	LT	R
Adjusted Flow Rate		1607	499	220	1563					443	113	396
Proportion of LT or RT	0.000	--	0.000	0.000	--	0.000		--		0.000	--	0.000

Saturation Flow Rate

Base Satflow		1900	1900	1900	1900					1900	1900	1900
Number of Lanes		3	1	1	3					1	1	1
f_w		1.000	1.000	1.000	1.000					1.000	1.000	1.000
f_{HV}		0.980	0.980	0.980	0.980					0.980	0.980	0.980
f_g		1.000	1.000	1.000	1.000					1.000	1.000	1.000
f_p		1.000	1.000	1.000	1.000					1.000	1.000	1.000
f_{bb}		1.000	1.000	1.000	1.000					1.000	1.000	1.000
f_a		1.000	1.000	1.000	1.000					1.000	1.000	1.000
f_{LU}		0.908	1.000	1.000	0.908					1.000	1.000	1.000
f_{LT}		1.000	--	0.950	1.000	--			--	0.950	1.000	--
Secondary f_{LT}			--	0.075	0.756	--			--			--

f_{RT}	--	1.000	0.850	--	1.000		--			--	1.000	0.850
f_{Lpb}		1.000	--	1.000	1.000	--			--	1.000	1.000	--
f_{Rpb}	--	1.000	1.000	--	1.000		--			--	1.000	1.000
Adjusted Satflow		5074	1583	1770	5074					1770	1863	1583
Secondary Adjusted Satflow			--	141	3836	--			--			--

CAPACITY AND LOS WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Capacity Analysis												
	EB			WB			NB			SB		
Lane Group		<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>					<i>L</i>	<i>LT</i>	<i>R</i>
Adjusted Flow Rate		1607	499	220	1563					443	113	396
Satflow Rate		5074	1583	1770	5074					1770	1863	1583
Lost Time		2.0	2.0	2.0	2.0					2.0	2.0	2.0
Green Ratio		0.45	0.45	0.58	0.58					0.32	0.32	0.32
Lane Group Capacity		2306	720	245	2952					563	593	504
v/c Ratio		0.70	0.69	0.90	0.53					0.79	0.19	0.79
Flow Ratio		0.32	0.32	0.10	0.31					0.25	0.06	0.25
Critical Lane Group		Y	N	Y	N					Y	N	N
Sum Flow Ratios	0.67											
Lost Time/Cycle	16.00											
Critical v/c Ratio	0.78											
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Lane Group		<i>T</i>	<i>R</i>	<i>L</i>	<i>T</i>					<i>L</i>	<i>LT</i>	<i>R</i>
Adjusted Flow Rate		1607	499	220	1563					443	113	396
Lane Group Capacity		2306	720	245	2952					563	593	504
v/c Ratio		0.70	0.69	0.90	0.53					0.79	0.19	0.79
Green Ratio		0.45	0.45	0.58	0.58					0.32	0.32	0.32
Uniform Delay d ₁		24.0	23.9	29.4	13.9					34.1	27.2	34.1
Delay Factor k		0.26	0.26	0.42	0.13					0.50	0.50	0.50
		0.9	2.9	32.0	0.2					10.6	0.7	11.7

Incremental Delay d_2												
PF Factor		1.000	1.000	1.000	1.000					1.000	1.000	1.000
Control Delay		24.9	26.8	61.4	14.1					44.7	27.9	45.8
Lane Group LOS		C	C	E	B					D	C	D
Approach Delay	25.3			19.9						43.2		
Approach LOS	C			B						D		
Intersection Delay	26.9			Intersection LOS						C		

FULL REPORT

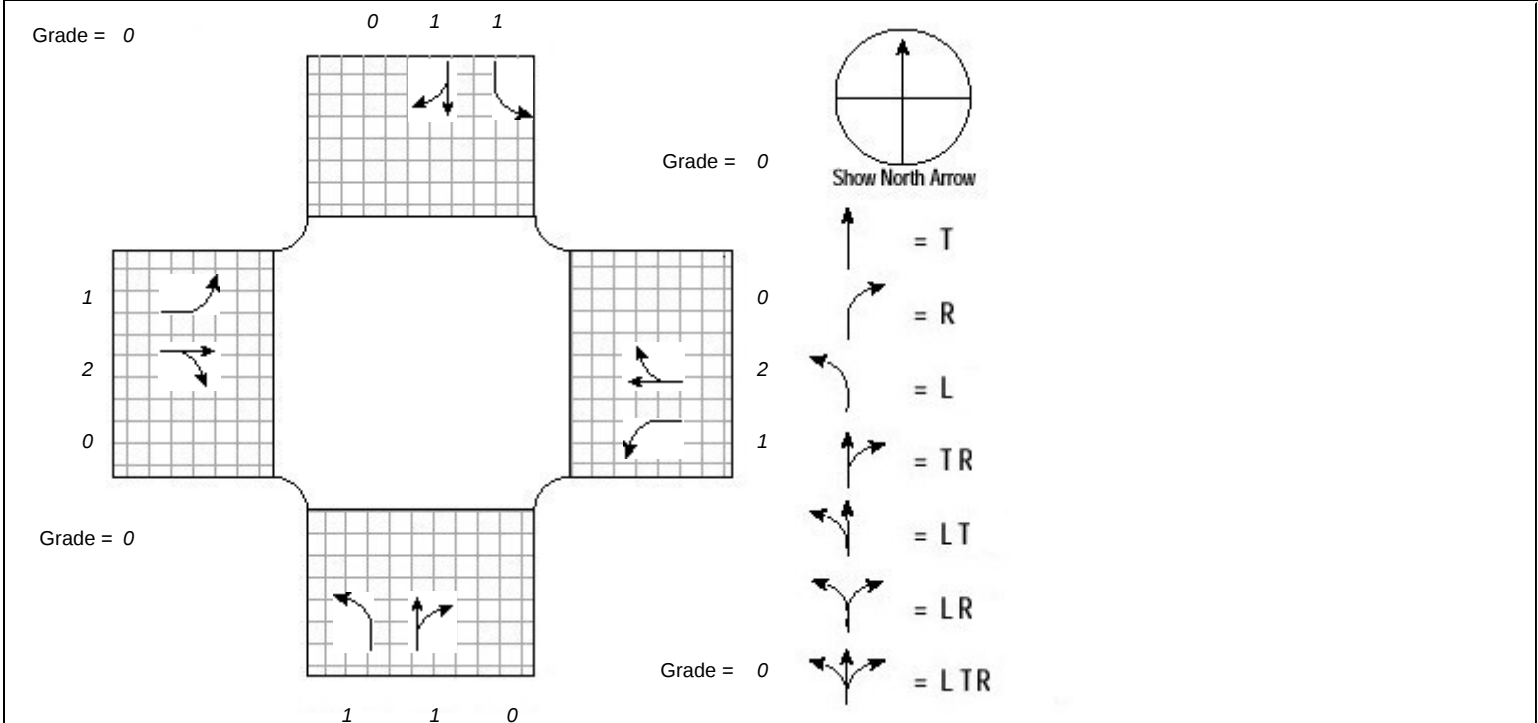
General Information	
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Analyst	LSB
Agency or Co.	Cathy Sweetapple & Associates
Date Performed	08/20/2008
Time Period	PM Peak Hour

Site Information

Intersection	SW 184 Street/SW 147 Avenue
Area Type	All other areas
Jurisdiction	Miami-Dade
Analysis Year	Future with Project

Intersection Geometry

[illegible][illegible]

Pedestrian Timing			3.2		3.2		3.2		3.2	
	EW Perm	02	03	04	NS Perm	06	07	08		
Timing	G = 51.0	G =	G =	G =	G = 50.0	G =	G =	G =		
	Y = 5	Y =	Y =	Y =	Y = 4	Y =	Y =	Y =		
Duration of Analysis (hrs) = 0.25					Cycle Length C = 110.0					

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	58	532	111	20	442	88	116	264	71	84	274	52
PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adjusted Flow Rate	61	560	117	21	465	93	122	278	75	88	288	55
Lane Group	L	TR		L	TR		L	TR		L	TR	
Adjusted Flow Rate	61	677		21	558		122	353		88	343	
Proportion of LT or RT	0.000	--	0.173	0.000	--	0.167	0.000	--	0.212	0.000	--	0.160
Saturation Flow Rate												
Base Satflow	1900	1900		1900	1900		1900	1900		1900	1900	
Number of Lanes	1	2	0	1	2	0	1	1	0	1	1	0
f_w	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_{HV}	0.980	0.980		0.980	0.980		0.980	0.980		0.980	0.980	
f_g	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_p	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_{bb}	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_a	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
f_{LU}	1.000	0.952		1.000	0.952		1.000	1.000		1.000	1.000	
f_{LT}	0.387	1.000	--	0.321	1.000	--	0.442	1.000	--	0.433	1.000	--
Secondary f_{LT}			--			--			--			--

f _{RT}	--	0.974		--	0.975		--	0.968		--	0.976	
f _{Lpb}	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--	1.000	1.000	--
f _{Rpb}	--	1.000		--	1.000		--	1.000		--	1.000	
Adjusted Satflow	721	3455		598	3458		824	1803		806	1818	
Secondary Adjusted Satflow			--			--			--			--

General Information

Capacity Analysis

Lane Group Capacity, Control Delay, and LOS Determination

PF Factor	1.000	1.000		1.000	1.000		1.000	1.000		1.000	1.000	
Control Delay	18.5	20.5		16.9	19.5		19.7	20.7		18.7	20.5	
Lane Group LOS	B	C		B	B		B	C		B	C	
Approach Delay	20.3			19.4			20.5			20.1		
Approach LOS	C			B			C			C		
Intersection Delay	20.1			Intersection LOS						C		

FULL REPORT	
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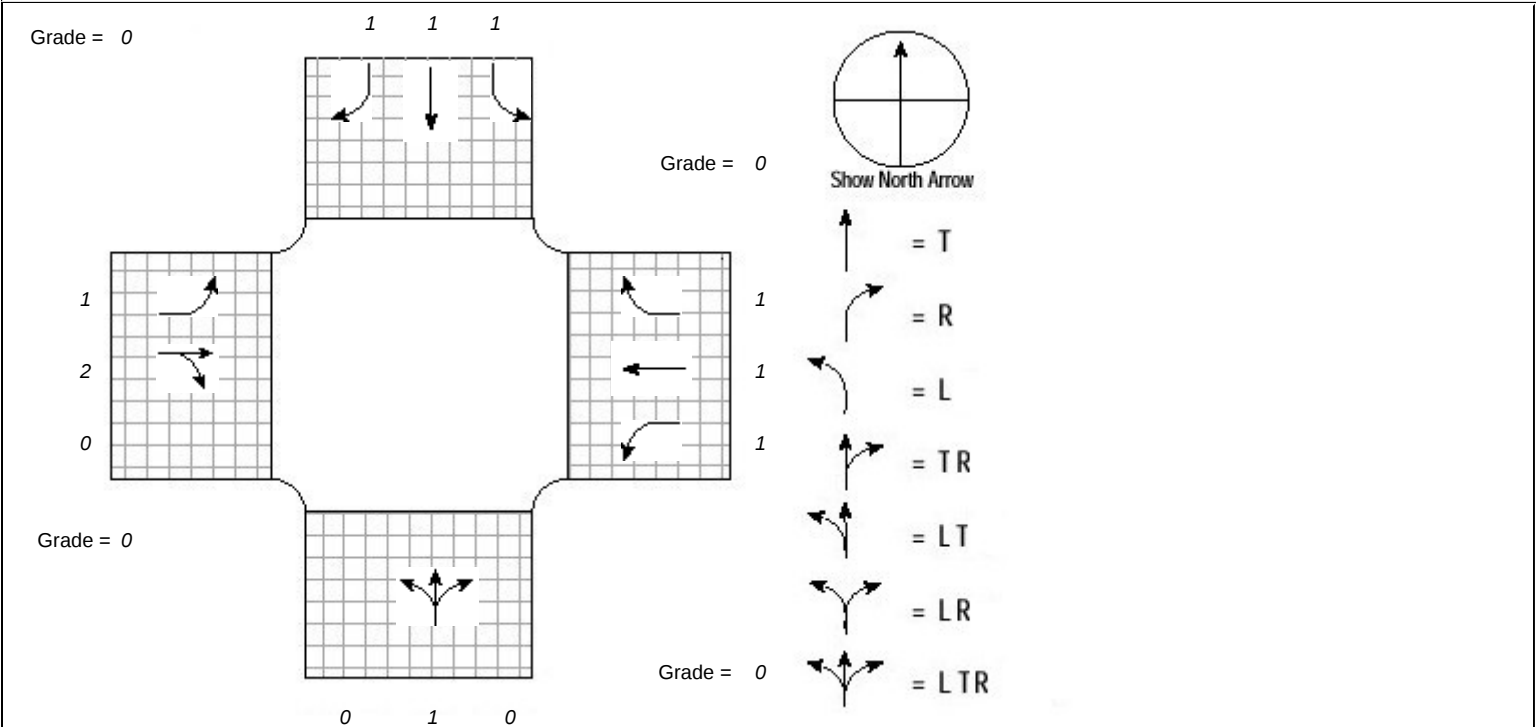
General Information	
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Analyst	LSB
Agency or Co.	Cathy Sweetapple & Associates
Date Performed	08/20/2008
Time Period	PM Peak Hour

Site Information

Intersection	SW 184 Street/SW 157 Avenue
Area Type	All other areas
Jurisdiction	Miami-Dade
Analysis Year	Future with Project

Intersection Geometry	
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Volume and Timing Input	
Volume	Timing
1000	1000
2000	2000
3000	3000
4000	4000
5000	5000
6000	6000
7000	7000
8000	8000
9000	9000
10000	10000

[illegible]

Pedestrian Timing			3.2		3.2		3.2		3.2	
	EW Perm	02	03	04	NS Perm	06	07	08		
Timing	G = 58.0	G =	G =	G =	G = 42.0	G =	G =	G =		
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =		
Duration of Analysis (hrs) = 0.25					Cycle Length C = 110.0					

VOLUME ADJUSTMENT AND SATURATION FLOW RATE WORKSHEET												
General Information												
Project Description <i>Parkland DRI</i>												
Volume Adjustment												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Volume	120	384	0	0	201	487	0	1	4	348	2	178
PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adjusted Flow Rate	126	404	0	0	212	513	0	1	4	366	2	187
Lane Group	L	TR		L	T	R		LTR		L	T	R
Adjusted Flow Rate	126	404		0	212	513		5		366	2	187
Proportion of LT or RT	0.000	--	0.000	0.000	--	0.000	0.000	--	0.800	0.000	--	0.000
Saturation Flow Rate												
Base Satflow	1900	1900		1900	1900	1900		1900		1900	1900	1900
Number of Lanes	1	2	0	1	1	1	0	1	0	1	1	1
f_w	1.000	1.000		1.000	1.000	1.000		1.000		1.000	1.000	1.000
f_{HV}	0.980	0.980		0.980	0.980	0.980		0.980		0.980	0.980	0.980
f_g	1.000	1.000		1.000	1.000	1.000		1.000		1.000	1.000	1.000
f_p	1.000	1.000		1.000	1.000	1.000		1.000		1.000	1.000	1.000
f_{bb}	1.000	1.000		1.000	1.000	1.000		1.000		1.000	1.000	1.000
f_a	1.000	1.000		1.000	1.000	1.000		1.000		1.000	1.000	1.000
f_{LU}	1.000	0.952		1.000	1.000	1.000		1.000		1.000	1.000	1.000
f_{LT}	0.604	1.000	--	0.498	1.000	--		1.000	--	0.754	1.000	--
Secondary f_{LT}			--			--			--			--

f_{RT}	--	1.000		--	1.000	0.850	--	0.892		--	1.000	0.850
f_{Lpb}	1.000	1.000	--	1.000	1.000	--		1.000	--	1.000	1.000	--
f_{Rpb}	--	1.000		--	1.000	1.000	--	1.000		--	1.000	1.000
Adjusted Satflow	1126	3547		927	1863	1583		1662		1405	1863	1583
Secondary Adjusted Satflow			--			--			--			--

General Information

Capacity Analysis

	EB			WB			NB			SB		
Lane Group	<i>L</i>	<i>TR</i>		<i>L</i>	<i>T</i>	<i>R</i>		<i>LTR</i>		<i>L</i>	<i>T</i>	<i>R</i>
Adjusted Flow Rate	126	404		0	212	513		5		366	2	187
Satflow Rate	1126	3547		927	1863	1583		1662		1405	1863	1583
Lost Time	2.0	2.0		2.0	2.0	2.0		2.0		2.0	2.0	2.0
Green Ratio	0.53	0.53		0.53	0.53	0.53		0.38		0.38	0.38	0.38
Lane Group Capacity	594	1870		489	982	835		635		536	711	604
v/c Ratio	0.21	0.22		0.00	0.22	0.61		0.01		0.68	0.00	0.31
Flow Ratio	0.11	0.11		0.00	0.11	0.32		0.00		0.26	0.00	0.12
Critical Lane Group	<i>N</i>	<i>N</i>		<i>N</i>	<i>N</i>	<i>Y</i>		<i>N</i>		<i>Y</i>	<i>N</i>	<i>N</i>
Sum Flow Ratios	0.58											
Lost Time/Cycle	10.00											
Critical v/c Ratio	0.64											

[illegible]

PF Factor	1.000	1.000		1.000	1.000	1.000		1.000		1.000	1.000	1.000
Control Delay	14.7	14.1		12.3	14.4	21.5		21.1		32.0	21.0	24.1
Lane Group LOS	B	B		B	B	C		C		C	C	C
Approach Delay	14.3			19.5			21.1			29.3		
Approach LOS	B			B			C			C		
Intersection Delay	21.0			Intersection LOS						C		